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Tadeusz Łagoda

Lifetime Estimation of Welded Joints



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Preface

In the paper the author attempts to assess the fatigue life of chosen welded joints. It focuses especially on chosen problems that accompany determination of the fatigue life of welded joints, taking into consideration the strain energy density parameter. Chapter 2 describes the welded joint as a stress concentrator. The state of stress and strain in the notch are described and theoretical and fatigue coefficients are indicated. The fatigue coefficient of the notch effect is estimated on the basis of fictitious radius in the notch root. Chapter 3 presents a model of fatigue life assessment under uniaxial stress state with statistical handling of data presented. The new energy model of fatigue life assessment, which rests upon the analysis of stress and strain in the critical plane, is described in detail in chapter 4. The principle of such a description is presented in the uniaxial as well as in biaxial state of loading. Chapter 5 contains the analysis of tests of four materials subjected to different loadings: cyclic, variable-amplitude with Gaussian distribution, and variable amplitude with Gaussian distribution and overloading for symmetric and pulsating loading. The analysis is based on the determined fatigue characteristics for all the considered materials. Chapter 6 shows the application of the model in the fatigue life assessment in the complex state of loading (bending with torsion of flange-tube and tube-tube joints) based on fatigue research of steel and aluminum welded joints carried out in well-known German centres. Proportional and out-ofproportional cyclic research are carried out. Additionally, the influence of various bending and torsion frequencies and proportional and out-ofproportional variable amplitude loadings are analysed.

Dealing with such a complicated problem as fatigue life of welded joints is requires a wide cooperation with other researchers and research centres. That is why I would like to express his gratitude to at least some of the people who contributed to the issue of this publication. This book is a result of my research work, considerations and discussions while my sixmonths stay at LBF Darmstadt, Germany, financed by NATO. Thus, I would like to thank all the workers of LBF, especially Prof. C.M. Sonsino [218] and Dr. M. Küppers for many discussions, access for their test results, laboratories and library while my visit, and for information sent in our correspondence. I was able to complete my book while my later two-week

stay in Darmstadt, financed by DAAD. In this book, I also used some data obtained from Technical University of Clausthal, Germany, namely from Prof. H. Zenner. I want to thank Prof. Zenner, Prof. A.Esderts and Mr. A.Ahmadi for their help while my one-week visit at TU of Clausthal, financed by CESTI.

I also used my experience obtained during my work with the postgraduate students at Opole University of Technology: Dr. Damian Kardas, Dr. Krzysztof Kluger, Ms Małgorzata Kohut, Dr. Paweł Ogonowski, Dr. Jacek Słowik and Ms Karolina Walat. I must also thank my co-workers from Opole University of Technology: Dr. Adam Niesłony, Dr. Aleksander Karolczuk, Dr. Roland Pawliczek, and especially Prof. Ewald Macha, also some other people not mentioned here. I would like to thank Ms Ewa Helleńska for translation of this book and some previous papers into English. Finally, I want to thank Prof. M. Skorupa and Prof. K. Rosochowicz for their suggestions and advice.

I wish to dedicate this book to my wife Bożena for supporting me in my research work as well as for her constant understanding and care.

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Notation

A ₅ C	contraction coefficient containing circumferential stresses in the root of the
E f	notch longitudinal modulus of elasticity (Young's modulus) frequency
G	shear modulus
$k(N_f)$	ratio of allowable stresses for bending and torsion for a given
K,	number of cycles N _f fatigue notch coefficient
K _f	theoretical notch coefficient
K _{ta}	theoretical notch coefficient for axial loading
K _{tb}	theoretical notch coefficient for bending
K _{tt}	theoretical notch coefficient for torsion
Nf	number of stress cycles up to fracture
m, m'	slope of fatigue S-N characteristic curve
Μ	moment
N	number of cycles
P r	force correlation coefficient
R	stress ratio
$R_{0.2}, R_{e}$	yield point
R _{m.}	tensile strength
S C(TT)	coefficient of multiaxiality, standard deviation
$S(T_{o})$	fatigue damage degree in observation time T_o
t T _N	time, thickness of sheet scatter band for life-time
\overline{T}_{N}	mean scatter band for life-time
T _o	observation time
Ŵ	strain energy density parameter
α	angle of the plane position
γ	shear strain
3	normal strain

φ pha	se displacement angle
v Pois	sson ratio
ρ radi	us in the welding notch root
σ norr	mal stress
τ shea	ar stress
0xyz xyz	co-ordinate system with the origin in the point 0

Indices

	annulity da
а	amplitude
af	fatigue limit
b	bending
cal	calculation
e	elastic
eq	equivalent
exp	experimental
f	fictitious
1	local
m	mean value
max	maximum value
min	minimum value
n	nominal
р	plastic
t	torsion
x, y, z w	directions of axes of the co-ordinate system weighed value

Functions

$$\operatorname{sgn}(x) = \begin{cases} 1 & \text{for } x > 0\\ 0 & \text{for } x = 0\\ -1 & \text{for } x < 0 \end{cases}$$
$$\operatorname{sgn}(x, y) = \frac{\operatorname{sgn}(x) + \operatorname{sgn}(y)}{2}$$

1 Introduction

The problem of determination of fatigue life of welded joints has been investigated for many years. As a result, there is a possibility to find solution of that issue in many publications. Typical handbooks concerning fundamentals of machine building are for example [33, 194], other books and monographs [45, 53, 199, 203, 204, 215, 216, 234 and others] or the latest work [44], to mention some. The problem has been also discussed in many journal publications and presented in conferences. Only few of publications have been cited in this paper. During the last 15 years, the in-depth analysis of the problem of fatigue life calculations has been presented in many books and other publications [40]. The author of this monograph refers to the most important of them [4, 5, 40, 73, 100, 166, 176, 233].

Correct design of welded joints seems to be very important, for example in transport facilities, including hoisting equipment [84, 167] where special safety regulations must be fulfilled, or in the structures with high pressure of a medium [186].

According to [176, 232], in order to define the fatigue life in welded joints, there are two basic approaches possible to determine calculation stresses: first – on the basis of the nominal stresses, and second – on the basis of the strictly local stresses determined in the potential point of crack initiation ("hot spot").

Analysis based on the nominal stresses is applicable in the situation where the considered element has been classified and when the stresses can be easily determined. In [233], Susmel and Tovo presented satisfactory results of many calculations of welded joints based on nominal stresses under constant-amplitude loading.

The hot spot method is recommended for the cases where the strains can be measured near the joint [42, 176], or if the strains can be calculated with the finite element method. In [31] Dang Van et al., on the basis of the analysis of more than 200 fatigue tests of different steels (low- and high-strength) and different geometries of welded joints, founded that fatigue life of welded joints, calculated on the basis of stresses determined with the hot spot method [177, 178], is not strongly influenced by types of the materials joined. It is observable particularly for a number of cycles greater than $5 \cdot 10^5$. In the case of a lower number of cycles, for

higher-strength materials, the permissible stresses are higher than for normal steels. It is important to draw attention to the fact that the notch coefficients for high-strength steels are greater than those for low-strength steels [224]. Thus, it is interesting to analyse relation between strength of normal steels and higher-strength steels in the local notation, i.e. including the theoretical notch coefficient. The safe fatigue life of the butt joints is higher than that of the fillet joints. Maddox [166] claims that good results can be obtained for nominal stresses but, in his opinion, the hot spot method should be developed in future. Principles of local stress determination according to the hot spot method are presented in Fig. 1.1. The local stresses can be defined from strains determined by extrapolation using two or three tensometers, or calculated with the finite element method.

If the local approach for welded joints subjected to multiaxial loading is applied, it is necessary to know the stress concentration for bending and torsion (K_{tb} , K_{tt}) at the fusion edge [165, 217, 220, 221]. Because of the fact that it is usually not possible to measure the actual radius of the fusion edge, in order to solve this problem, a suitable method is necessary. For welded joints subjected to uniaxial loading, the problem has been successfully solved owing to the application of so-called fictitious radius or, in other words, conventional radius [202, 208], based on the Neuber theory [173, 174]. Local methods for determination of fatigue life of welded joints under multiaxial fatigue were reviewed by Labesse-Jied [89]. The calculated fatigue lives of welded joints made of C45 steel and subjected to proportional and non-proportional radiom tension-compression with torsion loading were located in the scatter band of coefficient 4. The analysis was done on the basis of local stresses with plastic strains. The method using the conventional radius in the notch root can be applied for determination of the theoretical notch coefficient in

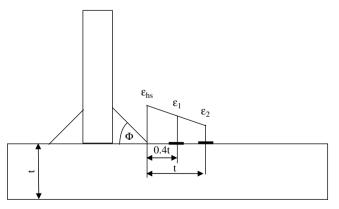


Fig. 1.1. Determination of strains with the hot-spot method

the case when the notch radius in the welded joint is small and tends to zero. In this paper, this method is considered for complex loading.

Similar to the Neuber's method based on the fictitious radius in the notch root, is the method proposed by Lawrence et al. (for example in [98]). In this model, determination of the maximum fatigue coefficient of stress concentration K_{fmax} is suggested. That value is determined for the critical radius in the notch root, equal to the critical value a* dependent on the material. It took the values from about 0.1 mm for welds made of high-alloy steels to 0.25 for low-alloy steels.

Another method of determination of geometric stresses, next applied for fatigue life calculations, was presented by Xiao and Yamada [246]. It was proposed by them to perform calculations with the use of stresses occurring 1 mm from the point of contact on the surface of the joined materials.

In [225] and [38, 47], Sonsino et al. claim that in practice damages should be accumulated on the assumption that the sum of damages according to the Palmgren-Miner hypothesis is D = 0.5. In [5], on the basis of the results of the tests under non-sinusoidal variable loading of the welded joints in bridges, at the drilling platforms or steel chimneys, it was observed that the sum of fatigue damages was D < 1. In [50], rough steel welded joints were subjected to variable-amplitude loading and it was found that damage accumulation in the considered joints varied about D = 1, and for the machined welded joints D = 0.33. In [96], Lahti found that the damage sum for variable-amplitude loading was less than 1. In [168], Mayer et al. stated that the experimental life was usually 3.5 times less than the life calculated according to the Palmgren-Miner fatigue damage sum is included in (0.33–1) according to various test results.

Standard recommendations referred to calculations of welded joints can be found in Eurocode 3 [38, 207], or the standards of the International Institute of Welding (IIW) [48] (for steels), and Eurocode 9 [39] (aluminium alloys).

In [12], typical fatigue diagrams for welded joints under axial loading (or bending) and torsion with constant inclination coefficients are presented (see Fig. 1.2).

In present paper two models of fatigue life estimation, based on stresses and the strain energy density parameter are presented.

For the uniaxial loading state (bending or axial loading), the model using local stresses was discussed. This model includes a value of the theoretical notch coefficient. As stated before, on the basis of [177, 178], it can be said that fatigue life of steel welded joints does not depend on a kind of material. In Chap. 3, there is a model of fatigue life assessment under uniaxial stress state with statistical handling of data presented. Chapter 5 contains the analysis of tests of four materials subjected to different

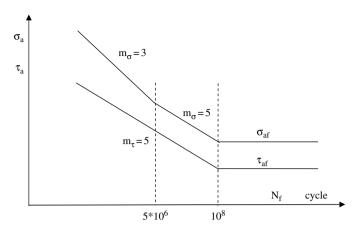


Fig. 1.2. Standard slopes of stress fatigue graphs for welded joints (σ_a –normal stress, τ_a – shear stress)

loading: cyclic, variable-amplitude with Gaussian distribution, and variable amplitude with Gaussian distribution and overloading for symmetric and pulsating loading. The analysis was based on the determined fatigue characteristics for all the considered materials.

Another approach is applied for the complex stress state. When the stress and strain tensors are determined for the welded joint, it is necessary to reduce the multiaxial loading state to the equivalent uniaxial state. For this purpose the fatigue effort criteria based on stress, strain, or the strain energy density parameter [115, 123, 136, 164] referred to the critical plane can be used. In Chap. 4, the energy model using the strain energy density parameter for complex loading was presented. This model includes both stresses and strains occurring in the material. In [156, 190, 193] it has been proved that in the case of great number of cycles the stress and energy models are the most appropriate for fatigue description, and for low numbers of cycles the strain and energy models are good. Thus, the energy model seems to be universal and it was verified many times in many papers concerning uniaxial loading [13, 14, 55, 59, 64, 65, 66, 83, 102, 103, 105, 106, 122, 136, 154, 179, 180] and complex loading [57, 83, 116, 118, 120, 124, 125, 130, 135, 140, 141, 142, 149, 150, 151, 152, 155, 187, 193]. In this paper, known results obtained for tube-tube and flange-tube joints under pure bending and torsion and their combination, in- or out-of-phase, and also for chosen steel welded joints under variable-amplitude loading [165, 217, 220, 221, 240, 241, 242, 243] and aluminum joints [86, 87, 88, 226] were evaluated. For analysis, some selected criteria based on the energy parameter for multiaxial fatigue were applied [32, 115, 123, 136, 164, 165, 214, 217, 220, 221].

2 Welded Joints as the Stress Concentrator

2.1 The Complex Stress State in the Notch

Complex stress concentration characterizes welded joints in which both geometrical and structural notches can be distinguished. In the case of geometrical notches under simple loading states, e.g. bending or axial loading, on the surface of the element in the notch root the plane stress state occurs. In round elements, apart from the nominal stress σ_x , the additional circumferential stress is also observed along the element. It can expressed by a formula

$$\sigma_{\rm v} = C\sigma_{\rm x},\tag{2.1}$$

where $0 \le C \le v$. For simplification it can be written as

$$C = \begin{cases} 0 & \text{for } K_t = 1 \\ 0 \div \nu & \text{for } K_t \in (1, 2), \\ \nu & \text{for } K_t > 2 \end{cases}$$
(2.2)

where K_t is the stress concentration ratio.

Analysing the results of calculations performed in LBF Darmstadt by Sonsino et al., the following equation can be formulated [193, 229]

$$C = \frac{1.84\nu}{K_t} (K_t - 1)^{1-\nu}.$$
 (2.3)

The coefficient C, defining the values of circumferential stresses depending on the stress concentration ratio is shown in Fig. 2.1. From the analysis of the figure it appears that the value of the coefficient C tends to the Poisson ratio, v, for K_t close to 2 (according to (2.2)).

It should be also noted that in the case of a sharp notch, the plane stress state is accompanied by the plane strain state (ε_x , $\varepsilon_y = 0$, ε_z). It results from the adoption of the elastic body model, the generalized Hooke's law, and $\sigma_y = v\sigma_x$ according to (2.1) and (2.2). For sharp notches, stress distributions for tension, bending and torsion are shown in Figs. 2.2, 2.3 and 2.4.

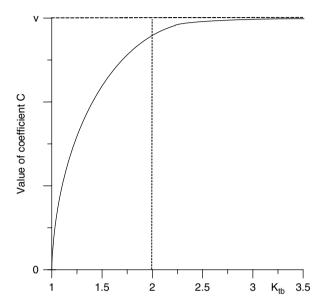


Fig. 2.1. Coefficient C versus theoretical stress concentration coefficient

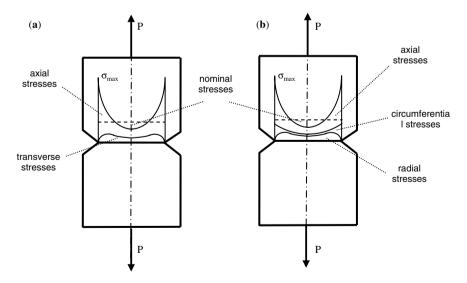


Fig. 2.2. Stress distributions in elements with sharp notches under tension: (a) flat element, (b) cylindrical element

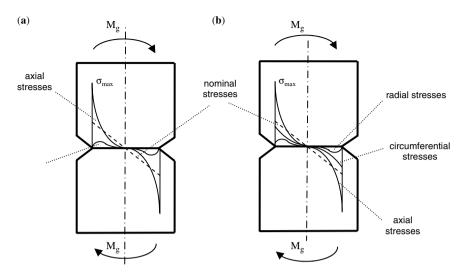


Fig. 2.3. Stress distributions in elements with sharp notches under bending: (a) flat element, (b) cylindrical element

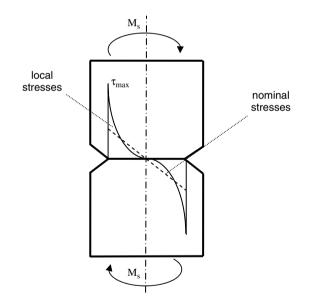


Fig. 2.4. Stress and strain distributions in the element with a sharp notch under torsion

Under tension and bending, stress distributions have been shown for flat and cylindrical elements. In both cases, on the notch root surface plane stress state is observable. In the case of flat elements, inside the material, the plane stress state occurs, and in cylindrical elements the spatial stress state is observed.

Stress and strain distributions in smooth and notched elements have been already analysed in [67, 108, 110, 135, 139, 153, 200, 201, 210, 212, 213]. They were used mainly in non-local methods of fatigue life assessment.

2.2 Theoretical Notch Coefficient

The theoretical notch coefficient is defined as

$$K_{t} = \frac{\sigma_{xx}^{e}}{\sigma_{xxn}}.$$
(2.4)

According to the Neuber rule, this coefficient can be expressed as the geometric mean from the stress and strain concentration coefficients

$$K_{t} = \sqrt{K_{\sigma}K_{\varepsilon}} , \qquad (2.5)$$

which are defined as

$$K_{\sigma} = \frac{\sigma_{xx}^{e-p}}{\sigma_{xxn}}$$
(2.6)

and

$$K_{\varepsilon} = \frac{\varepsilon_{xx}^{e-p}}{\varepsilon_{xxn}},$$
(2.7)

where ε_{xx}^{e-p} and σ_{xx}^{e-p} are the elastic-plastic strain and the stress in direction of x axis, respectively.

The notch coefficients can be determined in a numerical way, for example with the finite element method, appropriate monograms or suitable formulas, more or less complicated [171, 181, 183, 184 and 185]. There are also other models joining nominal and actual stresses [54], but they are not considered in this book. Up to the yield point, the following relation takes place

$$\mathbf{K}_{t} = \mathbf{K}_{\sigma} = \mathbf{K}_{\varepsilon}.$$
 (2.8)

For greater stresses, the known relation is valid [20, 24]

$$\mathbf{K}_{\sigma} \le \mathbf{K}_{t} \le \mathbf{K}_{\varepsilon},\tag{2.9}$$

(see Fig. 2.5).

Xiao and Yamada [246] point that the theoretical notch coefficient K_t for welded joints can be determined as a product of the weld geometry action K_w and the influence of structure change in the weld K_s , which can be expressed as

$$\mathbf{K}_{\mathrm{t}} = \mathbf{K}_{\mathrm{w}} \, \mathbf{K}_{\mathrm{s}}. \tag{2.10}$$

Influence of K_s changes in a welded joint was considered by Chen et al. in [28], and Cheng et al. [29], who tested specimens made of 1Cr–18Ni–9Ti steel under pure tension, pure torsion, and non-proportional tension with torsion. In the specimens tested there was no change of geometry in the joint, i.e. $K_w = 1$. The result scatters for welded joints were greater than those for the native metal. Under pure tension–compression and pure torsion, the fatigue strength of the weld material was less than that of the native material. Such a change was not observed under non-proportional tension with torsion. Thus, the coefficients K_s , including structure changes usually are not separately calculated, and it is assumed that

$$K_t = K_w.$$
 (2.11)

Whereas, the influence of the K_s coefficient is taken into account during the determination of fatigue notch performance coefficient. The theoretical notch performance coefficient K_t can be used for transformation of stress

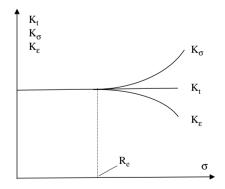


Fig. 2.5. Relation between theoretical notch coefficients and stress value

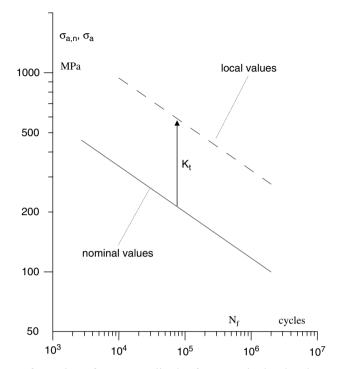


Fig. 2.6. Transformation of stress amplitudes from nominal to local system

amplitude values from nominal to local system, shown in Fig. (2.6) according to (2.4).

2.3 The Fatigue Notch Coefficient

The fatigue notch coefficient K_f [37, 219] is determined by comparison of stresses in smooth, σ_{sm} and notched σ_{not} elements

$$K_{f} = \frac{\sigma_{sm}}{\sigma_{not}}.$$
(2.12)

Interpretation of the coefficient K_f is shown in Fig. 2.7 [1, 2, 3, 119, 121, 135, 189, 190].

The fatigue notch coefficient is usually determined for 10^6 cycles, i.e. [97]

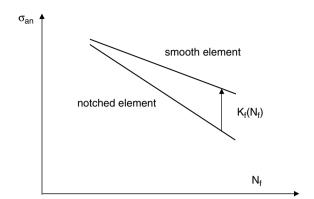


Fig. 2.7. Comparison of nominal stresses σ_{an} for smooth and notched elements

$$K_{f} = \frac{\sigma_{sm} \left(10^{6} \text{ cycles} \right)}{\sigma_{not} \left(10^{6} \text{ cycles} \right)}.$$
(2.13)

From Fig. 2.8 it appears that the fatigue notch coefficient increases as a number of cycles rises. Generally speaking, it can be stated that it is dependent on a number of cycles [20, 43, 197]

$$K_{f} = \frac{\sigma_{sm}(N_{f})}{\sigma_{not}(N_{f})}.$$
(2.14)

Let us derive a relationship [112]

$$K_{f}(N_{f}) = \left(\frac{N_{f}}{10^{3}}\right)^{\frac{\log[K_{f}(10^{6})]}{3}}.$$
(2.15)

According to this relationship, on the assumption that $N_f = 10^3$ cycles, the fatigue notch coefficient $K_f = 1$. However, it is usually given as a constant value for $N_f = 10^6$ cycles (2.13) (see Fig. 2.8). It can be written as

$$K_{f}(N_{f}) = K_{f}(10^{6}) \left(\frac{N_{f}}{10^{6}}\right)^{\frac{\log[K_{f}(10^{6})]}{3}}.$$
(2.16)

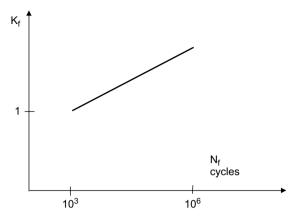


Fig. 2.8. Fatigue notch coefficient K_f versus number of cycles N_f

There are many models joining theoretical and fatigue notch coefficients in the following general form

$$\mathbf{K}_{\mathrm{f}} = \mathbf{f}(\mathbf{K}_{\mathrm{t}}). \tag{2.17}$$

Many papers, among these [175, 247] present relationships among different forms of (2.17) for various materials, types of notches and plastic strains occurring in the notch bottom.

2.4 The Fictitious Radius of the Welding Notch

For determination of notch coefficients the fictitious (conventional) radius in the notch root is applied. Determination of this radius results from stress averaging according to the Neuber's proposal [174]. It is assumed that the crack initiation is controlled by stress in the notch root averaged in a small volume of the material, in the point of the maximum stress occurrence. A suitable material parameter is a substitute microstructural length ρ^* . Stresses in the notch root must be averaged in the interval ρ^* in the direction normal to the surface along this length normal to the notch surface. Taking into account an actual radius in the notch root and the coefficient of multiaxiality s, the expression for the fictitious radius in the notch root is obtained

$$\rho_{\rm f} = \rho + s \rho^*. \tag{2.18a}$$

For the weld, the worst case can be assumed, i.e. the radius $\rho = 0$, which corresponds to the crack. Then the calculated fictitious radius is expressed as

$$\rho_{\rm f} = {\rm s} \ \rho^*. \tag{2.18b}$$

If the radius is known, it is possible to calculate the notch coefficient, components of the local stress tensor and the corresponding strains. The fictitious radius also depends on geometry of the specimen and a loading mode [159, 173, 174, 202, 208] – they should be taken into account under biaxial bending and torsion (see Table 2.1).

The fictitious notch coefficient ρ_f depends on the actual notch coefficient ρ , the substitute microstructural length ρ^* and the coefficient of multiaxiality s from Table 2.1 (according to the Neuber's proposal) resulting from the stress state multiaxiality in the notch root.

In [229, 230] and some other papers, the authors proposed to determine the substitute microstructural length * according to the following equation

$$\rho^* = \frac{\rho}{s} \left[\frac{(K_t - 1)^2}{(K_f - 1)^2} - 1 \right]$$
(2.19)

(see Fig. 2.9).

As it was said above, the zero notch radius, $\rho = 0$, is often assumed with $\rho^* = 0.4$ mm for welded steels, $\rho^* = 0.1$ mm for aluminium alloys [173, 174, 202, 208] and s = 2.5 for plane specimens, when the Huber-Mises-Hencky criterion is used. Then, the fictitious radius $\rho_f = 1$ mm for welded steels and $\rho_f = 0.25$ mm for aluminium is obtained, on the basis of which the

Loading	axial or bending		shearing or torsion
Specimen	plane	Round	-
Criterion			
Huber-Mises-Hencky	2.5	$5-2\nu+2\nu^2$	1
		$2 - 2\nu + 2\nu^2$	
Tresca	2	2-v	1
		1-v	
maximum normal stresses	2	2	1
Beltrami	2 - v	2 - v	1
		1-v	

Table 2.1. Coefficients of multiaxiality s according to Neuber [173, 174]

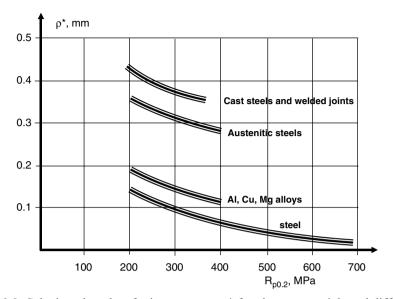


Fig. 2.9. Substitute lengths of microstructure ρ^* for chosen materials and different yield points

determination of the fatigue notch performance coefficient is possible. However, in the case of round specimens subjected to bending, on the assumption that the Poisson's number v=0.3, for welded steels the following formula is obtained

$$\rho_{\rm fb} = 0.4 \ \frac{5 - 2\nu + 2\nu^2}{2 - 2\nu + 2\nu^2} \ \rm{mm} = 1.16 \ \rm{mm}$$
(2.20)

under torsion the following expression is obtained

$$\rho_{\rm ft} = 0.4 \cdot 1 \,\,{\rm mm} = 0.4 \,\,{\rm mm}.$$
 (2.21)

for aluminium subjected to bending the following formula is obtained

$$\rho_{\rm fb} = 0.1 \ \frac{5 - 2\nu + 2\nu^2}{2 - 2\nu + 2\nu^2} \ \rm{mm} = 0.29 \ \rm{mm}$$
(2.22)

and for torsion

$$\rho_{\rm ft} = 0.1 \cdot 1 \,\,{\rm mm} = 0.1 \,\,{\rm mm}.$$
 (2.23)

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2.5 The Notch Coefficient with the Use of the Fictitious Notch Radius

Huther et al. [51] considered fillet joints and analysed influence of the angle of weld face inclination Θ within (30°–55°), and the radius in the notch root ρ within (0.5–3) mm on the fatigue limit. Geometry of such a joint is shown in Fig. 2.10. When the angle rises under stresses determined according to the nominal system, the fatigue limit decreases. The fatigue limit decreases also as the radius in the notch bottom ρ increases.

In [172], influence of the weld face inclination $(0^{\circ}-90^{\circ})$ and the transfer radius (0.6–0.9) mm on the theoretical notch coefficient was considered. Greater notch coefficients are obtained for smaller notch radii. For angles $(45^{\circ}-75^{\circ})$ stabilization of the notch coefficient is observed.

In the weld penetration zone the existence of the fictitious radius in the notch root can be assumed, keeping the same weld face inclination, and then it is possible to determine the theoretical notch coefficient K_t .

Thus, in order to calculate K'_{tb} and K'_{tt} from the fictitious radius in the notch root ρ_f in round specimens, it is necessary to chose separately the fictitious radii for welded elements subjected to bending ($\rho_{fb} = 1.16 \text{ mm}$) and torsion ($\rho_{ft} = 0.4 \text{ mm}$), and for elements made of aluminium alloy under bending ($\rho_{fb} = 0.29 \text{ mm}$) and torsion ($\rho_{ft} = 0.1 \text{ mm}$) when the constant angle Θ is kept.

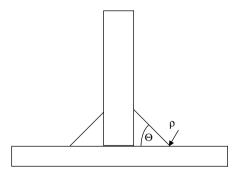


Fig. 2.10. Weld joint with the marked angle of weld face inclination Θ and the radius in the notch root ρ

3 The Stress Model for the Assessment of Fatigue Life Under Uniaxial Loading

3.1 Algorithm for the Assessment of Fatigue Life Under Uniaxial Loading State

Fatigue failure of machine and structure elements caused by service loading often occurs under random stress state. In such a situation, fatigue life is usually calculated with analytic methods or cycle counting methods. The analytic methods use spectral analysis of stochastic processes, and the cycle counting methods are based on numerical algorithms of cycle and halfcycle counting from histories of stress, strain or the energy parameter. The cycle counting methods include schematization of random loading histories, damage accumulation and then fatigue life calculation. Schematization of random histories includes counting of amplitudes and mean values of cycles and half-cycles occurring in the loading history.

In order to define fatigue life under random stress states, determination of the basic fatigue characteristic of the considered material is necessary. It is defined on the basis of cyclic fatigue tests. The basic characteristics for great number of cycles are the stress characteristics in the system $\sigma_a - N_f$, the so-called S–N characteristics. The first characteristic was elaborated by Wöhler [244] in 1860 in a single logarithmic system

$$\log N_{\rm f} = a + b\sigma_a. \tag{3.1}$$

In 1910, Basquin [16] proposed a characteristic that can be written in a double logarithmic system as

$$\log N_{f} = a + b \log \sigma_{a}. \tag{3.2}$$

It is necessary to point out that many authors meaning the Basquin characteristic call it the Wöhler curve. In 1914, Stromeyer [231] presented another proposal including the fatigue limit

$$\log N_{f} = a + b \log(\sigma_{a} - \sigma_{af}).$$
(3.3)

The next proposals were formulated by Corson in 1955 (see [195])

$$N_{f} = \frac{a}{\sigma_{a} - \sigma_{af}} \exp[-c(\sigma_{a} - \sigma_{af})]$$
(3.4)

and Bastenaire in 1974 (see [174, 195]

$$N_{f} = \frac{a}{\sigma_{a} - \sigma_{af}} \exp\left[-\left(\frac{\sigma_{a} - \sigma_{af}}{b}\right)^{c}\right].$$
(3.5)

Other models were discussed in papers by Palmgren [196], Weibull (1949), Stüssi (1955) and Bastenaire (1963) (see [173]), Kohout, Věchet (2001) [81]. However, the most frequently applied is the Basquin model expressed by (3.2), the so-called S–N fatigue curve.

Under uniaxial loading, fatigue life is calculated according to the algorithm shown in Fig. 3.1 and the stress model [17, 18, 94, 95, 127, 143, 144, 145, 148, 160, 164]. A similar algorithm for uniaxial loading has been proposed by Gołoś [43].

Stage 1

The input data for fatigue life calculations are strain $\epsilon(t)$ or stress $\sigma(t)$ histories, which can be obtained from:

• measurements of actual strains [41] or forces (strain gauges, extensometers, force gauges). Under uniaxial tension and on the assumption of a

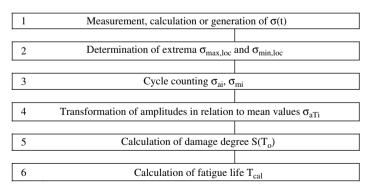


Fig. 3.1. Algorithm for determination of fatigue life under uniaxial random loading

perfectly elastic body, the relationship between the stress and strain histories can be written as

$$\sigma(t) = \mathrm{E}\varepsilon(t), \tag{3.6}$$

- previous numerical calculations [85] (FEM finite element method, BEM boundary element method, FDM finite difference method),
- computer generation of random sequences with shaped probabilistic characteristics corresponding to service conditions or the predicted states. Standard programs elaborated in some research centers can be used for this purpose. Some well-known standards are: WASH1 for loading simulation in drilling platforms [49, 205], Broad64 and MMMOD64 [6] (see also [5]) for drilling platforms, too, CARLOS [206] for car wheel loading, wind load [25]. Other possibilities of generation of signals have been presented, among others, in [26, 52, 85, 170, 198, 211, 236, 245, 248].

Stage 2

At this stage, extrema of the stress history are defined. Under random history, values of successive extrema are determined. This process includes observation of the derivative from the history and search of its monotonic changes, see Fig. 3.2. Figure 3.2 shows some exemplary determined local minima (2, 4, 6, 8) and extrema (1, 3, 5, 7, 9) in a course fragment.

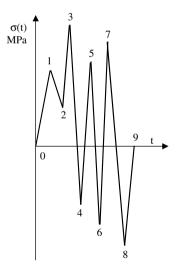


Fig. 3.2. Determination of local extrema

Stage 3

In the case of random histories with wide frequency bands, several cycle counting methods can be applied [80, 238]. The full cycle method in which half-cycles are not included, and the obtained life is usually overestimated. However, with the use of three other methods, i.e. the methods of range pairs. hysteresis loop and rain flow [10, 34, 35], the cycles, half-cycles and their mean values can be determinated. These three methods usually give similar results. In practice, the rain flow method (so-called envelope method) is most often applied. Its scheme is shown in Fig. 3.3. Envelopes are drawn from each local extremum (maximum or minimum). If the envelope has its beginning at the local minimum, it ends at the local maximum located opposite the local minimum, the value of which is lower than the initial minimum. The envelope beginning at the local minimum 0 ends at the local maximum 3 opposite the local minimum 4. The same procedure is applied when the envelope begins at the local maximum. Then it ends at the local minimum opposite the local maximum, the value of which is higher than the initial maximum. The envelope beginning at the local maximum 1 ends at the point 2 located opposite the local maximum 3. Half-cycles should be isolated from the determined cycles. The half-cycle of the largest span is included between the global maximum and global minimum (3 and 8). If the local minimum occurs as the first local minimum, the half-cycle is determined between this local minimum and the global maximum (0 and 3), and between this global maximum and the preceding local minimum (3 and 8). One more half-cycle is obtained between the global minimum and the last local maximum (8 and 9). The same procedure is applied when the beginning of the cycle is in the local maximum.

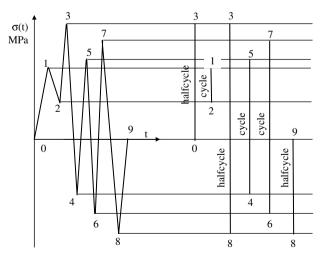


Fig. 3.3. Cycle counting with the rain-flow method [7, 34, 43, 80]

As it was mentioned above, the rain flow method, so-called envelope method, allows to define both cycles and half-cycles, which are determined by suitable envelopes (see Fig. 3.3). This method has been programmed and cycles are counted by the computer program. The amplitude σ_{ai} and the mean value σ_{mi} of a cycle or a half-cycle are determined each time.

Stage 4

At this stage, transformation of cycle amplitudes σ_{ai} takes place in relation to the occurring mean values σ_{mi} according to the general equation for the transformed amplitude, analysed in some previous papers [74, 75, 76, 113, 146, 147],

$$\sigma_{aTi} = f(\sigma_{ai}, \sigma_{mi}). \tag{3.7}$$

There are many models that take into account the influence of mean values. In this paper, the above transformations have not been widely presented because of the fact that in welded joints high residual stresses are often observed and then probable loading with the mean value occurring while a cycle do not influence the fatigue life.

Stage 5

There are many hypotheses of fatigue damage accumulation (stage 4) [235] (linear and nonlinear). The linear hypotheses proposed by Palmgren–Miner [169, 196], Haibach [47] and Serensen-Kogayev [209], Corten-Dolan [30], Liu-Zenner [99] are most frequently applied.

Damages can be accumulated according to the Palmgren–Miner hypothesis [169, 196], including amplitudes below the fatigue limit and the coefficient $a \le 1$

$$S_{PM}(T_{o}) = \begin{cases} \sum_{i=1}^{j} \frac{n_{i}}{N_{o} \left(\frac{\sigma_{af}}{\sigma_{ai}}\right)^{m}} & \text{for } \sigma_{ai} \ge a \cdot \sigma_{af} \\ 0 & \text{for } \sigma_{ai} < a \cdot \sigma_{af} \end{cases}$$
(3.8)

Haibach hypothesis [46]

$$S_{H}(T_{o}) = \begin{cases} \sum_{i=1}^{j} \frac{n_{i}}{N_{o} \left(\frac{\sigma_{af}}{\sigma_{ai}}\right)^{m}} & \text{for } \sigma_{ai} \ge \sigma_{af} \\ \\ \sum_{i=j}^{k} \frac{n_{i}}{N_{o} \left(\frac{\sigma_{af}}{\sigma_{ai}}\right)^{2m-p}} & \text{for } \sigma_{ai} < \sigma_{af} \end{cases}$$

$$(3.9)$$

where [223]:

p = 1 for steels and aluminium alloys, p = 2 for casts and sintered steels,

Serensen-Kogayev hypothesis [209]

$$S_{SK}(T_{o}) = \begin{cases} \sum_{i=1}^{j} \frac{n_{i}}{bN_{o} \left(\frac{\sigma_{af}}{\sigma_{ai}}\right)^{m}} & \text{for } \sigma_{ai} \ge a \cdot \sigma_{af} \\ 0 & \text{for } \sigma_{ai} < a \cdot \sigma_{af} \end{cases}$$
(3.10)

where:

$$b = \frac{\sum_{i=1}^{k} \sigma_{ai} t_{i} - a \cdot \sigma_{af}}{\sigma_{a \max} - a \cdot \sigma_{af}}$$
 for $b > 0.1$, (3.11)

is the Serensen-Kogayev coefficient, connected with a history character, and

$$t_i = \frac{n_i}{\sum_{i=1}^k n_i}$$
(3.12)

is frequency of occurrence of particular levels σ_{ai} in observation time T_0 , and σ_{af} is the general fatigue limit. The relationship (3.11) is valid if the following condition is satisfied $\frac{\sigma_{a \max}}{\sigma_{af}} > 1$ and $\frac{1}{\sigma_{a \max}} \sum_{i=1}^{k} \sigma_{ai} t_i > 0.5$.

Corten-Dolan hypothesis [30]

$$S_{CD}(T_{o}) = \begin{cases} \sum_{i=1}^{j} \frac{n_{i}}{N_{1} \left(\frac{\sigma_{a \max}}{\sigma_{ai}}\right)^{m'}} & \text{for } \sigma_{ai} \ge \sigma_{af} \\ 0 & \text{for } \sigma_{ai} < \sigma_{af} \end{cases}$$
(3.13)

where

m' = (0.8–0.9) m, N₁ = N_o
$$\left(\frac{\sigma_{af}}{\sigma_{a \max}}\right)^m$$
 (3.14)

Liu-Zenner hypothesis [99]

$$S_{LZ}(T_o) = \begin{cases} \sum_{i=1}^{j} \frac{n_i}{N_1 \left(\frac{\sigma_{a \max}}{\sigma_{ai}}\right)^{m'}} & \text{for } \sigma_{ai} \ge a \cdot \sigma_{af} \\ 0 & \text{for } \sigma_{ai} < a \cdot \sigma_{af} \end{cases}$$
(3.15)

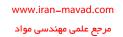
where

$$m' = \frac{m + m_i}{2}, \quad N_1 = N_o \left(\frac{\sigma_{af}}{\sigma_{a \max}}\right)^m, \quad (3.16)$$

and m_i is a slope of the fatigue curve S–N for fatigue crack initiation.

It is important to note that the models (3.12) and (3.15) act in a similar way as the Liu-Zenner model, however the Liu-Zenner model explains a new slope of the fatigue curve.

A model similar to the Serensen-Kogayev proposal was formulated for alloys of non-ferrous metals [58]. From calculations, b less or greater than 1 is obtained, depending on the mean-square weighed amplitude [107, 157]



$$S_{KL}(T_{o}) = \begin{cases} \sum_{i=1}^{j} \frac{n_{i}}{b' N_{o} \left(\frac{\sigma_{af}}{\sigma_{ai}}\right)^{m}} & \text{for } \sigma_{ai} \ge a \cdot \sigma_{af} \\ 0 & \text{for } \sigma_{ai} < a \cdot \sigma_{af} \end{cases}$$
(3.17)

where a coefficient including a damage degree $(D \neq 1)$ is determined. This coefficient characterizes the history and takes the form

$$b' = \frac{\sigma(N_f)}{\sigma_{aw}},$$
(3.18)

where

 σ_{aw} – stress amplitude for the given number of cycles, expressed by the following equation:

$$\sigma_{aw} = \left[\frac{\sum_{i} \sigma_{ai}^{2}}{\sum_{i} n_{i}}\right]^{\frac{1}{2}} \text{ for } \sigma_{ai} > a\sigma_{af}, \qquad (3.19)$$

 $\begin{aligned} \sigma_{ai} - stress \ amplitude, \\ n_i - a \ number \ of \ stress \ cycles \ with \ amplitude \ \sigma_{ai.} \end{aligned}$

where: σ_{aw} – stress amplitude for the given number of cycles.

In the discussed calculations, it was assumed that a number of cycles $N_{\rm f}$ was equal to $N_0,$ so:

$$\sigma(N_f) = \sigma_{af}.$$
(3.20)

Thus, when

$$\begin{split} \sigma_{aw} &> \sigma(N_f) \ \ \text{then} \ \ b' {<} 1 \\ \sigma_{aw} &= \sigma(N_f) \ \ \text{then} \ \ b' {=} 1. \\ \sigma_{aw} &< \sigma(N_f) \ \ \text{then} \ \ b' {>} 1 \end{split}$$

The damage degree changes depending on a level and a number of stress amplitudes. It decreases as the amplitudes increase. Figure 3.4 shows a

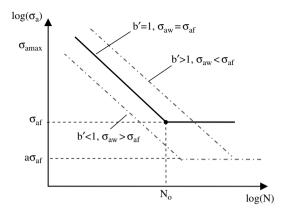


Fig. 3.4. Changes of the coefficient b' for $\sigma(N_f) = \sigma_{af}$

scheme of changes of the coefficient b' depending on the weighed amplitudes for the S–N curve.

The hypotheses (3.8, 3.9, 3.10, 3.13, 3.15, 3.17) can be written as one expression:

$$S(T_{o}) = \begin{cases} \sum_{i=1}^{J} \frac{n_{i}}{b * N * (\sigma_{af} / \sigma_{ai})^{m}} & dla \quad \sigma_{ai} \ge a\sigma_{af} \\ & , \quad (3.21) \\ h \sum_{i=j+1}^{k} \frac{n_{i}}{N * (\sigma_{af} / \sigma_{ai})^{(2m-p)}} & dla \quad \sigma_{ai} < a\sigma_{af} \end{cases}$$

where:

 $S(T_o)$ – material damage degree at time T_o according to (3.8, 3.9, 3.10, 3.13, 3.15) or (3.17),

 n_i – a number of cycles with amplitudes σ_{ai} in T_o ,

 $T_{o}-$ observation time $\,$ (for analysis of loadings with variable amplitudes a number of cycles in one block, N_{bloc} is assumed),

m – exponent of the S–N fatigue curve,

m' – modified slope coefficient for the S–N fatigue curve for Corten-Dolan (3.13) and Liu-Zenner (3.17) hypotheses, in another case m'=m,

 N_o – a number of cycles corresponding to the fatigue limit σ_{af} ,

 $N^* = N_1$ for Corten – Dolan (3.13) and Liu – Zenner (3.17), in another case $N^* = N_1$,

k - a number of class intervals of the amplitude histogram (j < k),

a – coefficient allowing to include amplitudes below σ_{af} in the damage accumulation process, (for Haibach (3.9) and Corten-Dolan (3.13) a = 1),

 b^* – coefficient including history character; for Serensen-Kogayev (3.10) $b^* = b$, for Kardas-Łagoda [57] (3.17) $b^* = b$, in other cases $b^* = 1$,

p – coefficient modifying the fatigue curve according to Haibach for amplitudes below the fatigue limit,

h – coefficient for the Haibach hypothesis (3.9) h =1 (for other hypotheses h = 0).

General forms are shown in Fig. 3.5, and method of damage accumulation according to the Palmgren–Miner rule is presented in Fig. 3.6.

As it was stated before, the assumption of linear summation of fatigue damage with modifications was proved many times during experiments

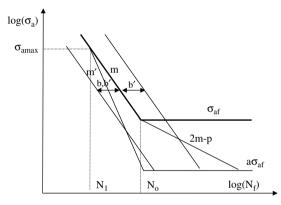


Fig. 3.5. Original Basquin fatigue curve and its modifications for fatigue damage accumulation

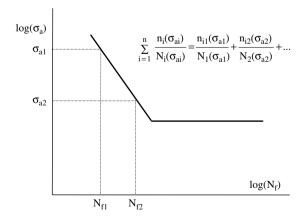


Fig. 3.6. A way of fatigue damage accumulation

under uniaxial loading being a stationary stochastic process of normal probability distribution.

Stage 6

After determination of a damage degree during observation time T_o according to a general form (3.21), fatigue life is determined

$$T_{cal} = \frac{T_o}{S(T_o)}.$$
(3.22)

After determination of the damage degree $S(N_{block})$ for a number of cycles N_{block} in a loading block according to the general formula (3.21), fatigue life is calculated according to the following equation

$$N_{cal} = \frac{N_{block}}{S(N_{block})}.$$
(3.23)

3.2 Statistic Evaluation

From references, for example from [96], it appears that for fatigue tests large scatters are typical. There are scatters of life under a given loading or scatters of loading (stresses, strains, the strain energy density parameter) under the given life.

From the paper by Lahti et al. [96] it appears that in the case of life of welded joints test results are included in the scatter band with the coefficient about 4 with probability 95%. It is defined as life scatters, i.e.

$$T_{\rm N} = N_{\rm cal}/N_{\rm exp},\tag{3.24}$$

or inverse of (3.24)

$$T'_{N} = 1/T_{N} = N_{exp}/N_{cal}.$$
 (3.25)

Ratios (3.22) and (3.25) can be called the scatter band with coefficient T_N . This scatter band varies depending on materials, loading level and mode, and it is included within the range from 1.5 for a low number of cycles, steel and notched specimens to 5 for a level close to the fatigue limit, cast iron or welded joints. The least scatters are obtained for tension where all the section area is equally subject to cracking (Fig. 3.7). A little greater

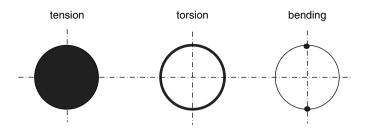


Fig. 3.7. The most loaded parts of the section under different simple loadings

scatters are often obtained under torsion (in the case of notches they are much greater), where the greatest stresses occur at the perimeter. The greatest scatters can be observed for bending, where only extreme fibres are loaded to a highest degree.

In [90, 91, 232], the error of life determination was related to the experimental life according to the following formula

$$E = \frac{\left| N_{cal} - N_{exp} \right|}{N_{exp}}.$$
 (3.26)

In [12] a standard deviation of logarithms of the calculated lives related to the experimental ones is presented

$$s_{N} = \sqrt{\frac{\sum_{i=1}^{n} (\log N_{cal} - \log N_{exp})^{2}}{n-1}}.$$
(3.27)

In [21], a scatter of the calculation results related to the experimental results is considered

$$T_{N} = \frac{T_{N10\%}}{T_{N90\%}},$$
(3.28)

where T_N is defined by (3.28), for 10% and 90% of probability of damage, respectively.

Other authors analyse the stress scatters.

Bellet et al. [19] compare calculated and experimental lives trying to assess efficiency of the models according to the following formula

$$E = \frac{\sigma_{exp}}{\sigma_{cal}}.$$
(3.29)

Another possibility of stress comparing is proposed by Sonsino and coauthors in many papers, for example [36, 219, 220, 222, 239] who apply the following equation:

$$T_{\sigma} = \frac{\sigma(P = 10\%)}{\sigma(P = 90\%)} \text{ for } N_{f} = \text{const.}$$
(3.30)

Such a formula includes only 80% calculating points.

Equation (3.30) for life can be written as:

$$T_{\rm N} = T_{\sigma}^{\rm m} \tag{3.31}$$

and in this case T_N is defined according to (3.31) as:

$$T_{\rm N} = \frac{N_{\rm f} \left(P = 10\% \right)}{N_{\rm f} \left(P = 90\% \right)}.$$
(3.32)

There are also models that include scatters of damage degrees analysis (see [27]), which is similar to (3.23)

$$E = \frac{\left| D_{cal} - D_{exp} \right|}{D_{exp}}.$$
(3.33)

During the analysis of life scatters the life ratios according to (3.24) or (3.25), or logarithms of life are usually used, according to

$$E = \log \frac{N_{exp}}{N_{cal}}$$
(3.34)

(see [11]).

The mean value of the considered quantity can be defined as

$$\overline{\mathbf{E}} = \frac{1}{n} \sum_{i=1}^{n} \mathbf{E}_i, \tag{3.35}$$

and the mean error of the mean value can be defined as

$$SE = \frac{s}{\sqrt{n}},$$
(3.36)

where n - a number of measurements, s - mean standard deviation.

For determination of the variance the following formula should used

$$s^{2} = \frac{1}{n-1} \sum_{i=1}^{n} (E_{i} - \overline{E})^{2}, \qquad (3.37)$$

and the standard deviation should be determined from variance (3.34)

$$\mathbf{s} = \sqrt{\mathbf{s}^2} \,. \tag{3.38}$$

In the case of material fatigue, the significance level is usually assumed at the minimum level $\alpha = 5\%$ or 10%, sometimes even 20%. Thus, the mean value should be included within the range

$$-t_{(n-1),\alpha/2}(SE) \le \overline{E} \le t_{(n-1),\alpha/2}(SE)$$
(3.39)

or

$$-t_{(n-l),\alpha/2}(s) \le \overline{E} \le t_{(n-l),\alpha/2}(s), \qquad (3.40)$$

where $t_{(n-1),\alpha/2}$ – constant from the t-Student's distribution for the mean value error SE – (3.36) or the population error s – (3.38).

Constant $t_{(n-1),\alpha/2}$ from the t-Student's distribution is determined for a half of the significance level $\alpha/2$ because of section of the normal distribution edges (see Fig. 3.8).

The mean scatter is determined from the following relationship

$$\overline{T}_{N} = 10^{\overline{E}}$$
(3.41)

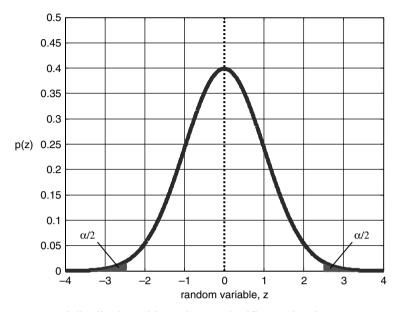


Fig. 3.8. Normal distribution with sections at significance level α

in the scatter band with the scatter coefficient T_N expressed as

$$T_{N} = 10^{t_{n-1}\alpha/2 \cdot s}.$$
 (3.42)

For the significance level $\alpha/2 = 2.5\%$ and n = 60 (often 20–30 measurements) the scatter band for all population is obtained. It is equal to two standard deviations (2s) (or maximum 2.2s for 20 measurements), which is often applied in tests [166] and corresponds to the scatter band with the coefficient 3. The scatters 3s correspond to a large significance level tending to zero but they are not applied in fatigue tests [9].

4 The Energy Model of Fatigue Life Assessment

For the complex loading state the energy model is being proposed. The model is based on the strain energy density parameter (SEDP) and analyses changes of stress (normal and shear) and strain (normal and shear) in the critical plane. It also distinguishes tension and compression.

4.1 The Energy Parameter Under Uniaxial Loading

The change of strain energy density, widely used in theory of plasticity, is also proposed as a parameter of the multiaxial fatigue analysis. Suitability of this parameter for description of fatigue processes seems to be promising, especially while formulation of thermal-elastic-plastic models of strain in the materials subjected to random thermomechanical loading. The models do not include a division of strain energy density into elastic and plastic parts, like in case of the parameters proposed by Smith–Watson–Topper (SWT) [214], Hoffman and Seeger [48], Bergman and Seeger [22]. In the elastic range, energy can be calculated from

$$W = \frac{1}{2}\sigma\varepsilon. \tag{4.1}$$

In the time domain, energy density can be expressed as

$$W(t) = \frac{1}{2}\sigma(t)\varepsilon(t). \tag{4.2}$$

When the equation is connected with the damage parameter

$$P_{SWT} = \sqrt{\sigma_{max} \varepsilon_a E}, \qquad (4.3)$$

where

$$\sigma_{\max} = \sigma_m + \sigma_a, \tag{4.4}$$

and when the mean value of stress is equal to zero

$$\sigma_{\max} = \sigma_a, \tag{4.5}$$

the strain energy density takes the form

$$W = \frac{\mathbf{P}_{\rm SWT}^2}{2\mathbf{E}}.\tag{4.6}$$

It should be pointed that parameter P_{SWT} has a dimension of stress. However, it is expressed as energy per a volume unit, i.e. MJ/m^3 .

Further modification of the considered parameter can be written as [22]

$$P = \sqrt{\left(k\sigma_{\rm m} + \sigma_{\rm a}\right)\varepsilon_{\rm a}E}$$
(4.7)

or for shear stresses and shear strains [48]

$$P_{\tau} = \sqrt{\tau_a \gamma_{\max} G} . \tag{4.8}$$

In order to distinguish tension and compression in a fatigue cycle, functions $sgn[\varepsilon(t)]$ and $sgn[\sigma(t)]$ should be substituted to (4.2):

$$W(t) = \frac{1}{4}\sigma(t)\varepsilon(t)\operatorname{sgn}[\varepsilon(t)] + \frac{1}{4}\sigma(t)\varepsilon(t)\operatorname{sgn}[\sigma(t)] =$$

= $\frac{1}{4}\sigma(t)\varepsilon(t)\operatorname{sgn}[\varepsilon(t)] + \operatorname{sgn}[\sigma(t)] = \frac{1}{2}\sigma(t)\varepsilon(t)\frac{\operatorname{sgn}[\varepsilon(t)] + \operatorname{sgn}[\sigma(t)]}{2}.$ (4.9)

A two-argument logical function is sensitive to the signs of variables, and it is defined as

$$sgn(x,y) = \frac{sgn(x) + sgn(y)}{2} = \begin{cases} 1 & \text{when } sgn(x) = sgn(y) = 1 \\ 0.5 & \text{when } (x = 0 \text{ and } sgn(y) = 1) & \text{or } (y = 0 \text{ and } sgn(x) = 1) \\ 0 & \text{when } sgn(x) = -sgn(y) & , \\ -0.5 & \text{when } (x = 0 \text{ and } sgn(y) = -1) & \text{or } (y = 0 \text{ and } sgn(x) = -1) \\ -1 & \text{when } sgn(x) = sgn(y) = -1 \end{cases}$$

$$(4.10)$$

where

$$\operatorname{sgn}(\mathbf{x}) = \begin{cases} 1 & \text{when } \mathbf{x} > 0 \\ 0 & \text{when } \mathbf{x} = 0 \\ -1 & \text{when } \mathbf{x} < 0 \end{cases}$$
(4.11)

After the substitution sgn(x,y) to (4.9) the following formula is obtained

$$W(t) = \frac{1}{2}\sigma(t)\varepsilon(t)\operatorname{sgn}[\sigma(t),\varepsilon(t)].$$
(4.12)

Equation (4.12) expresses positive and negative values of the strain energy density parameter in a fatigue cycle and it allows to separate energy (work) under tension from energy (work) under compression. If the parameter is positive, it means that the material is subjected to tension. If the parameter is negative, the material is subjected to compression with energy equal to this parameter for the absolute value. Equation (4.12) has another advantage: a course of the strain energy density parameter has the zero mean value, and the cyclic stress and strain have also the expected zero value, i.e. R = -1. Moreover, when stress or strain reaches zero, (4.9) is equal to zero, so sgn(x, y) = 0.5 does not occur. Thus, (4.10) can be written in the reduced form

$$\operatorname{sgn}(x, y) = \frac{\operatorname{sgn}(x) + \operatorname{sgn}(y)}{2} = \begin{cases} 1 & \text{when } \operatorname{sgn}(x) = \operatorname{sgn}(y) = 1 \\ 0 & \text{when } \operatorname{sgn}(x) = -\operatorname{sgn}(y) \\ -1 & \text{when } \operatorname{sgn}(x) = \operatorname{sgn}(y) = -1. \end{cases}$$
(4.13)

Figure 4.1a shows a course of the energy parameter according to (4.6), and the strain energy density parameter, taking into account signs of both stress and strain for an elastic body. If the signs of stresses and strains are not taken into account (Fig. 4.1b), a number of cycles with small ranges of energy parameters (Fig. 4.1b) is doubled and a non-zero mean value is obtained. This model is valid for R = -1.

If cyclic stresses and strains reach their maximum values, σ_a and ϵ_a , then the amplitude of maximum strain energy density parameter – according to (4.9)– is

$$W_a = 0.5\sigma_a \varepsilon_a. \tag{4.14}$$

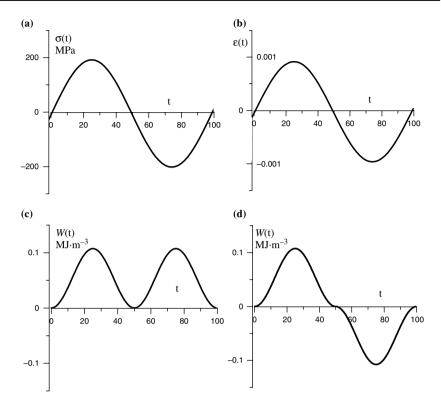


Fig. 4.1. Histories of cycle of stress, strain and strain energy density parameter -(4.2) and strain energy parameter including signs of stress and strain -(4.13)

Assuming - according to (4.9) – that W is the fatigue damage parameter, the standard characteristics of cyclic fatigue can be rescaled and it is possible to obtain a new characteristic (W_a-N_f) for low- and high-cycle fatigue. Under high-cycle fatigue, when the curve (σ_a-N_f) is applied, the axis σ_a should be replaced by W_a , where

$$W_a = \frac{\sigma_a^2}{2E}.$$
(4.15)

Using the Manson-Coffin-Basquin equation

$$\varepsilon_{a} = \varepsilon_{a}^{e} + \varepsilon_{a}^{p} = \frac{\sigma'_{f}}{E} (2N_{f})^{b} + \varepsilon'_{f} (2N_{f})^{c}$$

$$(4.16)$$

and (4.14), we obtain the strain energy density parameter

$$W_{a} = \frac{\sigma_{a}}{2} \left[\frac{\sigma'_{f}}{E} (2N_{f})^{b} + \varepsilon'_{f} (2N_{f})^{c} \right].$$

$$(4.17)$$

Equation (4.17) gives a new description of fatigue history, the curve (W_a $-\,N_f$), i.e.

$$W_{a} = \frac{(\sigma'_{f})^{2}}{2E} (2N_{f})^{2b} + 0.5\varepsilon'_{f} \sigma'_{f} (2N_{f})^{b+c}.$$
 (4.18)

Under high-cycle fatigue, (4.18) reduces to the following simple form

$$W_{a} = \frac{(\sigma'_{f})^{2}}{2E} (2N_{f})^{2b} = \frac{(\sigma'_{f})^{2}}{2E} (2N_{f})^{b'}.$$
 (4.19)

After finding the logarithm, further reduction takes place and the following formula is obtained

$$\log N_{\rm f} = A' - m' \log W_{\rm a}, \tag{4.20}$$

where:

$$A' = -\frac{1}{b'} \left[\log \frac{(\sigma'_{f})^{2}}{2E} + b' \log 2 \right],$$
(4.21)

$$\mathbf{m'} = -\frac{1}{\mathbf{b'}},\tag{4.22}$$

$$b'=2b$$
. (4.23)

Finally, the following expression is obtained

$$m' = \frac{1}{2b} = \frac{m}{2}.$$
 (4.24)

Under high-cycle fatigue, control of stress and strain are very close, especially in the case of cyclically stable materials. The constants A' and m' in (4.20) are determined from the fatigue curve S–N, by simple rescaling (4.18), determined on the basis of tests under controlled strain.

According to (4.15), replacing the stress amplitude by fatigue strength σ_{af} for a given fatigue life, the strain energy density parameter at the fatigue limit level is obtained

$$W_{af} = \frac{\sigma_{af}^2}{2E}.$$
(4.25)

Introducing the characteristic for shear strains

$$\gamma_{a} = \frac{\tau'_{f}}{G} (2N_{f})^{b_{\tau}} + \gamma'_{f} (2N_{f})^{c_{\tau}}$$
(4.26)

a new strain characteristics expressed by the parameter of shear strain energy density is obtained

$$W_{a} = \frac{(\tau'_{f})^{2}}{2G} N_{f}^{2b_{\tau}} + \frac{1}{2} \tau'_{f} \gamma'_{f} N_{f}^{b_{\tau}+c_{\tau}}$$
(4.27)

or, in the case of elastic strains

$$\log N_{\rm f} = A'_{\tau} - m'_{\tau} \log W_{\rm a}, \tag{4.28}$$

Figure 4.2 shows random histories of stress, strain and the parameter of normal strain energy density. From this figure it appears that in the case of the strain energy density parameter and neglecting signs of stresses and strains (Fig. 4.2c) the frequency band extension is obtained, and – in consequence – counting a greater number of cycles with mean values different from zero under the generated zero mean values of stresses and strains (Fig. 4.2a and b). On the other hand, using the strain energy density parameter and including signs of stresses and strains (Fig. 4.2d), it is possible to obtain a history with the zero mean value without extension of the frequency band, like for cyclic loading (Fig. 4.1).

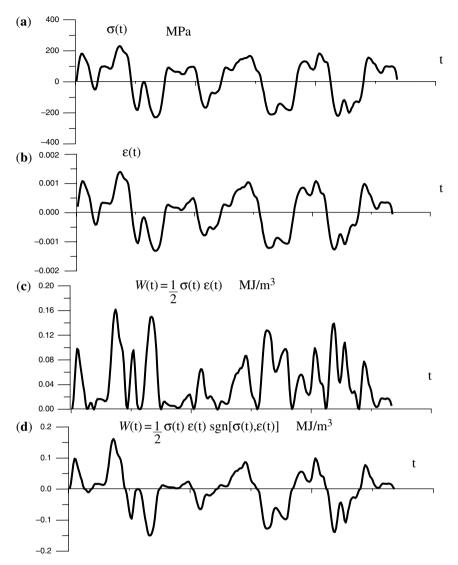


Fig. 4.2. Random history of stress, strain, strain energy density and normal strain energy density parameter

4.2 The Energy Parameter Under Multiaxial Loading

Analogous to the case of uniaxial loading state presented in Sect. 4.1 [115, 120, 123,124, 131, 136, 164, 188], the proposed generalized energy criterion is based on the analysis of stresses and the corresponding strains in the

critical plane, taking their signs into account. These criteria were formulated on the basis of previous considerations and papers [15, 82, 104, 111, 115, 117, 123, 129, 133, 134, 135, 137, 191, 192, 193]. The criteria valid under stress concentration, i.e. including the complex stress state under uniaxial loading, are presented below.

4.2.1 The Generalized Criterion of the Parameter of Normal and Shear Strain Energy Density Parameter in the Critical Plane

In the case of the proposed damage parameter under stress concentration the same assumptions as those for smooth elements can be made.

Fatigue cracking is caused by the part of strain energy density corresponding to work of the normal stress $\sigma_{\eta}(t)$ on the normal strain $\varepsilon_{\eta}(t)$, i.e. $W_{\eta}(t)$, and work of the shear stress $\tau_{\eta s}(t)$ on the shear strain $\varepsilon_{\eta s}(t) = 0.5\gamma_{\eta s}(t)$ in direction \overline{s} on the plane with normal $\overline{\eta}$, i.e. $W_{\eta s}(t)$;

- Direction s on the critical plane coincides with the mean direction of maximum shear strain energy density W_{nsmax}(t);
- 2. In the limit state, material effort is defined by the maximum value of linear combination of energy parameters $W_{\eta}(t)$ and $W_{\eta s}(t)$, where energy satisfies the following equation under multiaxial random loading

$$\max_{t} \{\beta W_{\eta s}(t) + \kappa W_{\eta}(t)\} = Q$$
(4.29)

or

$$\max_{t} \{ W(t) \} = Q, \tag{4.30}$$

where β is the constant for a particular form of (4.29), and κ and Q are material constants determined from sinusoidal simple fatigue tests.

The left sides of (4.29) and (4.30) can be written as maximum at time W(t) and they should be understood as 100% quantile of a random variable W. If the maximum value W(t) exceeds Q, then damage accumulation and failure take place. The random process W(t) can be interpreted as a stochastic process of the material fatigue strength. Positions of unit vectors

 $\overline{\eta}$ and \overline{s} are determined with the weight function method, variance method or damage accumulation method [138].

Selection of constants β , κ and Q in (4.29), and the assumed position of the critical plane leads to particular cases of the generalized criterion.

In special forms of the criterion a complex stress state should be assumed on the surface of the notched element. This is also necessary in the case of states under simple loading, such as bending or axial loading (tension-compression).

The equivalent value of the strain energy density parameter is a linear combination of energy density of normal and shear strains. Participation of particular energies in the damage process depends on the coefficients β and κ . In each case the scalar product of vectors is $\overline{\eta} \circ \overline{s} = 0$, where the vectors are defined as:

$$\overline{\eta} = \hat{l}_{\eta}\overline{i} + \hat{m}_{\eta}\overline{j} + \hat{n}_{\eta}\overline{k}, \qquad (4.31)$$

$$\overline{\mathbf{s}} = \hat{\mathbf{l}}_{\mathbf{s}} \,\overline{\mathbf{i}} + \hat{\mathbf{m}}_{\mathbf{s}} \,\overline{\mathbf{j}} + \hat{\mathbf{n}}_{\mathbf{s}} \,\overline{\mathbf{k}} \,. \tag{4.32}$$

The general criterion (4.29) can be written as

$$\max_{t} \left\{ \beta W_{\eta s}(t) + \kappa W_{\eta}(t) \right\} = W_{af}, \qquad (4.33)$$

From (4.33) the equation for the equivalent strain energy density parameter can be derived

$$W_{eq}(t) = \beta W_{\eta s}(t) + \kappa W_{\eta}(t), \qquad (4.34)$$

where:

$$W_{\eta s}(t) = 0.5\tau_{\eta s}(t)\varepsilon_{\eta s}(t)\operatorname{sgn}[\tau_{\eta s}(t),\varepsilon_{\eta s}(t)], \qquad (4.35)$$

$$W_{\eta}(t) = 0.5\sigma_{\eta}(t)\varepsilon_{\eta}(t)\operatorname{sgn}\left[\sigma_{\eta}(t),\varepsilon_{\eta}(t)\right]$$
(4.36)

and sgn[x,y] is given by (4.13).

Finally, the following expression is obtained

$$W_{eq}(t) = \frac{\beta}{2} \tau_{\eta s}(t) \varepsilon_{\eta s}(t) \operatorname{sgn}\left[\tau_{\eta s}(t), \varepsilon_{\eta s}(t)\right] + \frac{\kappa}{2} \sigma_{\eta}(t) \varepsilon_{\eta}(t) \operatorname{sgn}\left[\sigma_{\eta}(t), \varepsilon_{\eta}(t)\right].$$
(4.37)

Special cases of criterion (4.37) are discussed in the next chapters.

4.2.2 The Criterion of Maximum Parameter of Shear and Normal Strain Energy Density on the Critical Plane Determined by the Normal Strain Energy Density Parameter

The critical plane is defined by the normal strain energy density parameter. It is assumed that $Q = W_{af}$ and the critical plane with normal $\overline{\eta}$ is determined by normal loading, and position of the vector \overline{s} is determined by one of the directions defined by the given shear loading.

From the strain and stress states for pure torsion, tension-compression or bending and under constant-amplitude loading, it is possible to derive relationships coupling the coefficients β and κ .

According to (4.34), the amplitude of the equivalent strain energy can be written as

$$W_{aeq} = \beta W_{a\eta s} + \kappa W_{a\eta}. \tag{4.38}$$

Under only normal loading the following expression is obtained

$$W_{aeq} = W_{a\eta} = \frac{\sigma_{axx}^2}{2E} (1 - \nu C),$$
 (4.39)

where C is given by (2.3), and in the case of the sharp notch C = v.

For pure torsion on the plane of maximum tension, particular values of strain energy density are

$$W_{a\eta s} = 0 \tag{4.40}$$

and

$$W_{a\eta} = \frac{\sigma_{axx}^2}{2E} (1 - \nu C) \tag{4.41}$$

Introducing (4.39) - (4.41) to (4.38) subsequent expression is obtained

$$\kappa = 1. \tag{4.42}$$

For bending on the plane of maximum tension, the same values like for torsion are obtained, so it is not possible to determine the coefficient β with an analytical method. This coefficient can be assorted depending on a material after non-proportional tests. It could be done for constant-amplitude fatigue tests with phase shift $\pi/2$. The final criterion form (4.34) takes the form

 $W_{eq}(t) = \beta W_{ns}(t) + W_{n}(t).$ (4.43)

4.2.3 The Criterion of Maximum Parameter of Shear and Normal Strain Energy Density in the Critical Plane Determined by the Shear Strain Energy Density Parameter

In this case, the critical plane is determined by the parameter of shear strain energy density. It is assumed that the critical plane with normal $\overline{\eta}$ and tangent \overline{s} is defined as the mean position of one of two planes where the maximum shear strain energy density occurs. As in previous case, the equivalent parameter of strain energy density is determined from (4.34). Next, it is possible to write the amplitude of equivalent strain energy as a sum of parameters of normal and shear strain energy densities with weight coefficients β and κ .

Analysing the stress and strain state for pure torsion, tensioncompression or bending under constant-amplitude loading, the relationships coupling the coefficients β and κ can be determined.

For pure torsion on the maximum shear plane, particular values of the strain energy density parameter are

$$W_{a\eta} = 0, \qquad (4.44)$$

$$W_{a\eta s} = 0.5\tau_{axy}\varepsilon_{axy} = 0.25\tau_{axy}\gamma_{axy} = 0.25\frac{\tau^2_{axy}}{G} = \frac{\tau^2_{axy}(1+\nu)}{2E}$$
(4.45)

Introducing (4.41), (4.44), (4.45) into (4.38), the following formula is obtained

$$\frac{\sigma_{axx}^2(1-\nu C)}{2E} = \beta \frac{\tau_{axy}^2(1+\nu)}{2E}.$$
(4.46)

After transformation the following expression is obtained

$$\beta = \frac{\sigma_{axx}^{2}(1-\nu C)}{\tau_{axy}^{2}(1+\nu)}.$$
(4.47)

After introduction of

$$k = \frac{\sigma_{axx}^2}{\tau_{axy}^2},$$
(4.48)

the following expression can be obtained

$$\beta = \frac{k(1 - \nu C)}{(1 + \nu)}.$$
(4.49)

Similar calculations can be performed for pure bending or tensioncompression on the maximum shear plane. Particular stresses and strains on the chosen plane are

$$\sigma_{axn} = \frac{\sigma_{axx} + \sigma_{ayy}}{2} = \frac{\sigma_{axx}(1+C)}{2}, \qquad (4.50)$$

$$\tau_{axy} = \frac{\sigma_{axx} - \sigma_{ayy}}{2} = \frac{\sigma_{axx} (1 - C)}{2}, \qquad (4.51)$$

$$\gamma_{axy} = \frac{\varepsilon_{axx} - \varepsilon_{ayy}}{2} = \frac{\sigma_{axx} - \nu \sigma_{ayy} - \sigma_{ayy} + \nu \sigma_{axx}}{2E} = \frac{\sigma_{axx} (1 - \nu C - C + \nu)}{2E}, \quad (4.52)$$

$$\varepsilon_{axn} = \frac{\varepsilon_{axx} + \varepsilon_{ayy}}{2} = \frac{\sigma_{axx} - \nu \sigma_{ayy} + \sigma_{ayy} - \nu \sigma_{axx}}{2E} = \frac{\sigma_{axx} (1 - \nu C + C - \nu)}{2E} .$$
(4.53)

Thus, the subsequent formula is obtained

$$W_{a\eta} = 0.5\sigma_{axn}\varepsilon_{axn} = 0.5\frac{\sigma_{axx}(1+C)}{2} \cdot \frac{\sigma_{axx}(1-\nu C+C-\nu)}{2E} = \frac{\sigma_{axx}^{2}(1-\nu)(1+C)^{2}}{8E}$$
(4.54)

$$W_{a\eta s} = 0.5 \tau_{axy} \gamma_{axy} = 0.5 \frac{\sigma_{axx}(1-C)}{2} \cdot \frac{\sigma_{axx}(1-vC-C+v)}{2E} = \frac{\sigma_{axx}^2(1+v)(1-C)^2}{8E}.$$
(4.55)

Introducing (4.41), (4.54) and (4.55) into (4.38) the following expression is obtained

$$\frac{\sigma_{axx}^2}{2E}(1-\nu C) = \beta \frac{\sigma_{axx}^2(1+\nu)(1-C)^2}{8E} + \kappa \frac{\sigma_{axx}^2(1-\nu)(1+C)^2}{8E}.$$
 (4.56)

After suitable transformations, the relation between the coefficients β and κ is obtained

$$(1-\nu C) = \beta \frac{(1+\nu)(1-C)^2}{4} + \kappa \frac{(1-\nu)(1+C)^2}{4}.$$
 (4.57)

Thus

$$\kappa = \frac{4(1-\nu C) - \beta(1-C)^2(1+\nu)}{(1+C)^2(1-\nu)}.$$
(4.58)

After introduction of the determined value of the coefficient $\boldsymbol{\beta}$ the expression takes the form

$$\kappa = \frac{(4 - k(1 - C)^2)(1 - \nu C)}{(1 - \nu)(1 + C)^2}.$$
(4.59)

If values of the coefficients $\beta\,$ and $\kappa\,$ are taken into account, (4.37) takes the form

$$W_{eq}(t) = \frac{k(1-\nu C)}{(1+\nu)} W_{\eta s}(t) + \frac{(4-k(1-C)^2)(1-\nu C)}{(1-\nu)(1+C)^2} W_{\eta}(t).$$
(4.60)

If C = 0 for smooth elements, (4.60) for such elements takes the following form [155, 156, 187, 188]

$$W_{eq}(t) = \beta W_{\eta s}(t) + \frac{4 - \beta(1 + \nu)}{1 - \nu} W_{\eta}(t).$$
(4.61)

Relation between the coefficients β and κ and the ratio of normal stress amplitude (bending or tension-compression) to shear stress (torsion) for $\nu = 0.3$ is shown in Fig. 4.3.

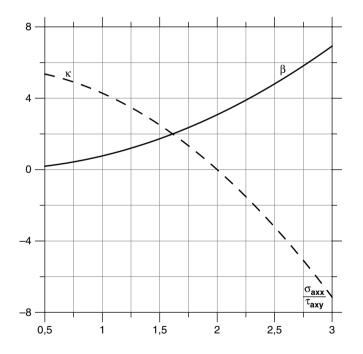


Fig. 4.3. Coefficients β and κ for different amplitude ratios σ_{axx}/τ_{axy}

4.3 Algorithm for Fatigue Life Assessment

Under multiaxial loading, fatigue life is calculated according to the general algorithm shown in Fig. 4.4. Particular stages of this algorithm are discussed below. Special attention was paid to the stages which were not previously presented in the algorithm describing procedures for uniaxial tension-compression in the stress description.

The proposed algorithms of fatigue life assessment for elements of machines and structures under multiaxial random loading have not been well verified so far and many laboratory tests should be performed in order to determine the ranges of their applicability in design calculations. Some of these algorithms have been already verified for chosen materials [132] and it is necessary to consider if they can be applied to calculation of fatigue life for other materials. Suitable selection of the multiaxial fatigue criteria seems be a very important problem. From a review of literature it appears that many proposed criteria of multiaxial fatigue are based on the critical plane. How should the critical plane be defined? The model presented below is based on the energy criteria that were presented earlier in this work.

Figure 4.5 shows the general algorithm of fatigue life determination using the criterion of shear and normal strain energy density parameter on the critical plane [115, 123, 136, 164].

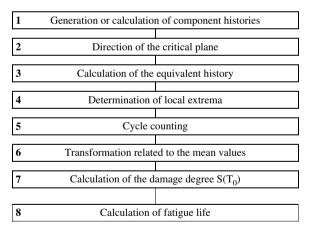


Fig. 4.4. Algorithm for fatigue life calculations under multiaxial random loading when the expected critical plane position is determined with the damage accumulation method

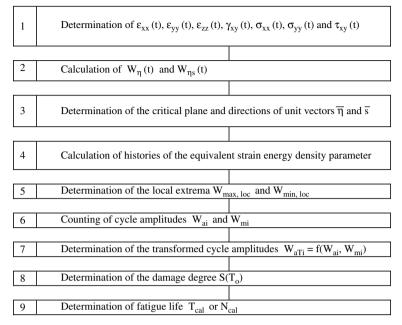


Fig. 4.5. Stages of fatigue life calculations under multiaxial loading according to the energy model

Stage 1

Histories of strain, ε (t) and / or stress, σ (t) are the input data for fatigue life calculations. As it was said in Sect. 3.1, they can come from measurements of actual strains or forces and moments, from previous numerical calculations or computer generation of random sequences with the formed probabilistic characteristics corresponding to service conditions or expected states. Such generations and simulation calculations are especially important under complex loading, when experimental tests are very expensive [92, 93, 126,128, 237].

General models are based on the full matrix of stresses

$$\sigma_{ij} = \begin{bmatrix} \sigma_{xx} & \tau_{xy} & \tau_{xz} \\ \tau_{xy} & \sigma_{yy} & \tau_{yz} \\ \tau_{xz} & \tau_{yz} & \sigma_{zz} \end{bmatrix}.$$
(4.62)

In the considered case, the plane stress state occurs in the notch root

$$\sigma_{ij} = \begin{vmatrix} \sigma_{xx} & \tau_{xy} & 0 \\ \tau_{xy} & \sigma_{yy} & 0 \\ 0 & 0 & 0 \end{vmatrix}.$$
 (4.63)

It is important to note that in the critical place of stress concentration (in the considered edge of the weld) the local biaxial stress state with components σ_{xx} , σ_{yy} and τ_{xy} can be observed. These local components are obtained by nominal values $\sigma_{xx,n}(t)$ and $\tau_{xy,n}(t)$, and particular theoretical stress concentration factors by action of the fatigue notch

$$\sigma_{xx}(t) = K_{tb}\sigma_{xx,n}(t), \qquad (4.64)$$

$$\sigma_{yy}(t) = C\sigma_{xx}(t) \tag{4.65}$$

and

$$\tau_{xy}(t) = K_{tt} \tau_{xy,n}(t).$$
(4.66)

These stresses correspond to nominal strains which can be determined from the following relationships under elastic conditions:

$$\varepsilon_{xx,n}(t) = \frac{\sigma_{xx,n}(t)}{E},$$
(4.67)

$$\varepsilon_{yy,n}(t) = -\nu \frac{\sigma_{xx,n}(t)}{E}, \qquad (4.68)$$

$$\varepsilon_{zz,n}(t) = -\nu \frac{\sigma_{xx,n}(t)}{E}, \qquad (4.69)$$

$$\varepsilon_{xy,n}(t) = \frac{\gamma_{xy,n}(t)}{2} = \frac{\tau_{xy,n}(t)}{2G}.$$
(4.70)

Generally, strains can be written as the full matrix of strains

$$\varepsilon_{ij} = \begin{bmatrix} \varepsilon_{xx} & \varepsilon_{xy} & \varepsilon_{xz} \\ \varepsilon_{xy} & \varepsilon_{yy} & \varepsilon_{yz} \\ \varepsilon_{xz} & \varepsilon_{yz} & \varepsilon_{zz} \end{bmatrix}.$$
(4.71)

In the considered case, the plane stress state in the notch root can be observed, so for strains

$$\varepsilon_{ij} = \begin{bmatrix} \varepsilon_{xx} & \varepsilon_{xy} & 0\\ \varepsilon_{xy} & \varepsilon_{yy} & 0\\ 0 & 0 & \varepsilon_{zz} \end{bmatrix}.$$
 (4.72)

For local strains

$$\varepsilon_{xx}(t) = (1 - C\nu) K_{tb} \frac{\sigma_{xx,n}(t)}{E}, \qquad (4.73)$$

$$\varepsilon_{yy}(t) = (C - \nu) K_{tb} \frac{\sigma_{xx,n}(t)}{E}, \qquad (4.74)$$

$$\varepsilon_{zz}(t) = -(C + \nu)K_{tb} \frac{\sigma_{xx,n}(t)}{E}, \qquad (4.75)$$

$$\varepsilon_{xy}(t) = \frac{\gamma_{xy}(t)}{2} = \frac{K_{tt}\gamma_{xy,n}(t)}{2} = K_{tt}\frac{\tau_{xy,n}(t)}{2G}.$$
(4.76)

For the sharp notch C = v the $\varepsilon_{yy} = 0$ is obtained – it results from (4.74) and means that the plane state is accompanied with the plane strain state. On the surface on the notch edges (or weld edges) the stresses $\sigma_{\eta}(t)$ and $\tau_{\eta s}(t)$ can be calculated according to the following equations:

$$\sigma_{\eta}(t) = \hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + \hat{n}_{\eta}^{2} \sigma_{zz}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \sigma_{xy}(t) + 2 \hat{l}_{\eta} \hat{n}_{\eta} \sigma_{xz}(t) + 2 \hat{m}_{\eta} \hat{n}_{\eta} \sigma_{yz}(t)$$
(4.77)

and

$$\begin{aligned} \tau_{\eta s}(t) &= \hat{l}_{\eta} \hat{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + \hat{n}_{\eta} \hat{n}_{s} \sigma_{zz}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \sigma_{xy}(t) \\ &+ \left(\hat{l}_{\eta} \hat{n}_{s} + \hat{l}_{s} \hat{n}_{\eta} \right) \sigma_{xz}(t) + \left(\hat{m}_{\eta} \hat{n}_{s} + \hat{m}_{s} \hat{n}_{\eta} \right) \sigma_{yz}(t). \end{aligned}$$

$$(4.78)$$

The strains $\varepsilon_n(t)$ and $\varepsilon_{ns}(t)$ are calculated from:

$$\varepsilon_{\eta}(t) = \hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + \hat{n}_{\eta}^{2} \varepsilon_{zz}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \varepsilon_{xy}(t) + 2 \hat{l}_{\eta} \hat{n}_{\eta} \varepsilon_{xz}(t) + 2 \hat{m}_{\eta} \hat{n}_{\eta} \varepsilon_{yz}(t)$$

$$(4.79)$$

and

$$\begin{aligned} \varepsilon_{\eta s}(t) &= \hat{l}_{\eta} \hat{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + \hat{n}_{\eta} \hat{n}_{s} \varepsilon_{zz}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \varepsilon_{xy}(t) + \\ \left(\hat{l}_{\eta} \hat{n}_{s} + \hat{l}_{s} \hat{n}_{\eta} \right) \varepsilon_{xz}(t) + \left(\hat{m}_{\eta} \hat{n}_{s} + \hat{m}_{s} \hat{n}_{\eta} \right) \varepsilon_{yz}(t). \end{aligned}$$

$$(4.80)$$

Because of the biaxial stress state on the surface of edges of the notch (or the weld), the stresses $\sigma_{\eta}(t)$ and $\tau_{\eta s}(t)$, and the strains $\varepsilon_{\eta}(t)$ and $\varepsilon_{\eta s}(t)$ -according to (4.77)–(4.78) – can be calculated from:

$$\sigma_{\eta}(t) = \hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \sigma_{xy}(t), \qquad (4.81)$$

$$\tau_{\eta s}(t) = \hat{l}_{\eta} \hat{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta}\right) \sigma_{xy}(t), \qquad (4.82)$$

$$\varepsilon_{\eta}(t) = \hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + \hat{n}_{\eta}^{2} \varepsilon_{zz}(t) + 2\hat{l}_{\eta}\hat{m}_{\eta} \varepsilon_{xy}(t), \qquad (4.83)$$

$$\varepsilon_{\eta s}(t) = \hat{l}_{\eta} \hat{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + \hat{n}_{\eta} \hat{n}_{s} \varepsilon_{zz}(t) + (\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta}) \varepsilon_{xy}(t).$$
(4.84)

Stage 2

If the histories of stresses and strains in a plane defined by direction cosines are known, it is possible to define histories of the strain energy density parameter, using (4.81)–(4.84). It should be noted that in the further part of strain in direction of axis y, ε_{yy} are neglected as they are not significant because stresses in this direction are zero, $\sigma_{yy}=0$.

All the mentioned energy criteria of multiaxial fatigue – (4.43) and (4.61) – use the normal, W_{η} , and shear, $W_{\eta s}$, strain energy density parameter.

The normal strain energy density parameter according to (4.36), (4.81) and (4.83) can be written as:

$$\begin{split} W_{\eta}(t) &= 0.5 \left[\hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \tau_{xy}(t) \right] \\ &\left[\hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \varepsilon_{xy}(t) \right] \\ &sgn \left\{ \left[\hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \tau_{xy}(t) \right], \\ &\left[\hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \varepsilon_{xy}(t) \right] \right\}, \end{split}$$
(4.85)

where stresses $\sigma_{xx}(t)$, $\sigma_{yy}(t)$, $\tau_{xy}(t)$ and strains $\varepsilon_{xx}(t)$, $\varepsilon_{yy}(t)$, $\gamma_{xy}(t)$ are the local values according to (4.66), (4.73), (4.74) and (4.76), respectively.

The shear strain energy density parameter – according to (4.35), (4.82) and (4.84) – takes the form:

$$\begin{split} W_{\eta s}(t) &= 0.5 \left[\hat{l}_{\eta} \bar{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \tau_{xy}(t) \right] \cdot \\ &\left[\hat{l}_{\eta} \bar{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \varepsilon_{xy}(t) \right] \cdot \\ &sgn \left\{ \left[\hat{l}_{\eta} \bar{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \tau_{xy}(t) \right] , \\ &\left[\hat{l}_{\eta} \bar{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \varepsilon_{xy}(t) \right] \right\}, \end{split}$$

$$(4.86)$$

where the stresses $\sigma_{xx}(t)$, $\sigma_{yy}(t)$, $\tau_{xy}(t)$ and the strains $\varepsilon_{xx}(t)$, $\varepsilon_{yy}(t)$, $\gamma_{xy}(t)$ are local values, according to (4.66), (4.73), (4.74) and (4.76).

Stage 3

In the algorithm for fatigue life assessment, proper determination of the expected position of the critical plane in the point of the maximum material effort is very important. The stress and strain states in the material belong to the basic factors determining this plane position. Its position is defined by the direction cosines $\hat{l}_n, \hat{m}_n, \hat{n}_n$ ($n = \eta$, s) of unit vectors $\overline{\eta}$ i \overline{s} occurring in the fatigue criteria, where $\overline{\eta}$ is perpendicular, and \overline{s} is tangent to the critical plane (Fig. 4.6), i.e.

$$\overline{\eta} \circ \overline{s} = 0. \tag{4.87}$$

The following three methods are proposed for determination of the expected position of the critical position of fatigue fracture [109, 132, 161, 162]:

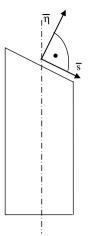


Fig. 4.6. Determination of the versors $\overline{\eta}$ and \overline{s} defining the critical plane

A – the weight function method, presented in [163]. In this method, instantaneous values of angles $\alpha_n(t)$, $\beta_n(t)$, $\gamma_n(t)$ are averaged. These angles determine instantaneous positions of principal axes of strains or stresses with special weight functions. Modifications of this method apply Euler angle averaging and they can be found in [56, 61, 62, 69, 70, 71, 72].

B - the maximum variance method, applied in [109, 132]. In the variance method it is assumed that the planes where variance of the equivalent history according to the chosen fatigue criterion reaches its maximum, are critical for the material. This method does not require much time for calculations, but statistic parameters of the strain (or stress) tensor components should be known. The variance method can be effective if the stress tensor components are stationary and ergodic stochastic processes with the same statistic character of loading.

C – *the method of damage accumulation*, discussed in [63, 67, 68, 101, 120, 133, 142]. Here, fatigue damages are accumulated in many planes of the given material point and the plane where the damage degree is maximum (i.e. where fatigue life is minimum) is selected. In the case when the assumed direction of the critical plane coincides with the criterion applied for fatigue life calculation, not only the expected critical plane direction is obtained but life as well. This method is more and more applied at present.

In the case of the method of damage accumulation and the variance method, their success depends on selection of a proper fatigue criterion



and a step of angle change discretization. The optimisation methods accelerating determination of the expected position of the critical plane are also applied. The critical plane and the fracture plane should be, however, distinguished. They are not always the same planes. Figure 4.7 shows the cracking models. In model I, normal stresses are responsible for fatigue cracking, in models II and III shear stresses cause cracking. There are materials where both shear and normal stresses are responsible for cracking. Then, it can be defined as the mixed model. The elastic-brittle materials crack according to model I, and the elastic-plastic materials – according to models II or III.

The model of fatigue damage accumulation used in this paper was verified many times for stress, strain and energy models.

Direction cosines $\hat{l}_{\eta}, \hat{m}_{\eta}, \hat{l}_{s}, \hat{m}_{s}$ of vectors $\hat{\eta}$ and \hat{s} , occurring in formulas for the normal or shear strain energy density parameter, (4.85) and (4.86), are under the plane stress state defined by one angle α in the following relationships:

$$\hat{l}_{\eta} = \cos \alpha, \quad \hat{m}_{\eta} = \sin \alpha, \quad \hat{l}_{s} = -\sin \alpha, \quad \hat{m}_{s} = \cos \alpha.$$
 (4.88)

Under random or variable-amplitude loading, a damage degree is calculated according to the algorithm shown in Fig. 4.8, but it is necessary to assume a suitable criterion of normal or shear strain energy density parameter, (4.85) or (4.86). From the previous considerations [156] it appears that the criterion assuming the plane determined by the maximum parameter of normal strain energy density, as the critical plane is valid for cast iron, i.e. a cast brittle material. Whereas, the criterion defined on the plane of the maximum parameter of shear strain energy density should be applied in life calculations for steels and non-ferrous metal alloys.

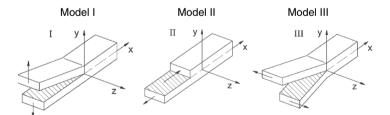


Fig. 4.7. Models of fatigue cracking [23]

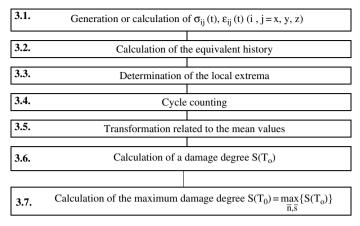


Fig. 4.8. Algorithm for determination of the critical plane direction by the normal or shear strain energy density parameter with the method of damage accumulation

The above algorithm becomes simplified under constant-amplitude loading. In such a case, it is necessary to determine the energy parameter history only for one cycle (one period T). If the normal strain energy density parameter is applied for determination of the critical plane, $W_{\eta}(t)$ for one cycle only should be calculated. Then, amplitude of the normal strain energy density parameter W_{na} is

$$W_{eq,a} = W_{\eta a} = \max_{T, \alpha} W_n(t, \alpha).$$
(4.89)

Equation (4.89) defines a position of the critical fracture plane expressed by $\alpha_0 = \alpha$. The critical plane is defined by the maximum equivalent amplitude of the normal strain energy density parameter $W_{eq,a}$.

If the shear strain energy density parameter is applied for determination of the critical plane, it is necessary to calculate $W_{\eta s}(t)$ for one cycle. Then, amplitude of the shear strain energy density parameter is $W_{\eta sa}$, and the shear strain energy density parameter is

$$W_{eq,a} = W_{\eta sa} = \max_{T,\alpha} W_{ns}(t,\alpha).$$
(4.90)

As in the case of the normal strain energy density parameter, Eq. (4.90) defines a position of the critical fracture plane expressed as $\alpha_0 = \alpha$. The critical

plane is defined by the maximum equivalent amplitude of the shear strain energy density parameter $W_{eq.a}$.

Stage 4

When the critical plane position is determined, it is necessary to determine the equivalent parameter of strain energy density on the critical plane.

In the case of the critical plane defined by the shear strain energy density parameter (4.86), the equivalent strain energy density parameter (4.61) can be defined as

$$\begin{split} W_{eq}(t) &= 0.5 \left[\hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \tau_{xy}(t) \right] \\ &= \left[\hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \varepsilon_{xy}(t) \right] \\ &= sgn \left\{ \left[\hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \tau_{xy}(t) \right] \right\} \\ &= \left\{ \hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \varepsilon_{xy}(t) \right] \right\} \\ &+ 0.5\beta \left[\hat{l}_{\eta} \bar{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \tau_{xy}(t) \right] \\ &= \left[\hat{l}_{\eta} \bar{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \varepsilon_{xy}(t) \right] \\ &= sgn \left\{ \left[\hat{l}_{\eta} \bar{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \tau_{xy}(t) \right] \right\} \\ &= \left[\hat{l}_{\eta} \bar{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \varepsilon_{xy}(t) \right] \right\} . \end{split}$$

In the case of the critical plane defined by the normal strain energy density parameter (4.85), the equivalent strain energy density parameter (4.43) can be expressed as

$$\begin{split} W_{eq}(t) &= 0.5 \frac{4 - \beta(1 + \nu)}{1 - \nu} \left[\hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \tau_{xy}(t) \right] \\ &\left[\hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \varepsilon_{xy}(t) \right] \\ &sgn \left\{ \left[\hat{l}_{\eta}^{2} \sigma_{xx}(t) + \hat{m}_{\eta}^{2} \sigma_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \tau_{xy}(t) \right], \\ &\left[\hat{l}_{\eta}^{2} \varepsilon_{xx}(t) + \hat{m}_{\eta}^{2} \varepsilon_{yy}(t) + 2 \hat{l}_{\eta} \hat{m}_{\eta} \varepsilon_{xy}(t) \right] \right\} \\ &+ 0.5\beta \left[\hat{l}_{\eta} \bar{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \tau_{xy}(t) \right] \cdot \\ &\left[\hat{l}_{\eta} \bar{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + \left(\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta} \right) \varepsilon_{xy}(t) \right] \cdot \end{split}$$

$$sgn\left\{ \begin{bmatrix} \hat{l}_{\eta} \bar{l}_{s} \sigma_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \sigma_{yy}(t) + (\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta}) \tau_{xy}(t) \end{bmatrix}, \\ \begin{bmatrix} \hat{l}_{\eta} \bar{l}_{s} \varepsilon_{xx}(t) + \hat{m}_{\eta} \hat{m}_{s} \varepsilon_{yy}(t) + (\hat{l}_{\eta} \hat{m}_{s} + \hat{l}_{s} \hat{m}_{\eta}) \varepsilon_{xy}(t) \end{bmatrix} \right\},$$
(4.92)

where β is defined by (4.49), and for sharp notches (C=v) the following expression is obtained

$$\beta = \mathbf{k}(1+\mathbf{v}) \tag{4.93}$$

and after introduction of (4.48) into (4.93) it takes the following form

$$\beta = \left(\frac{\sigma_{axx}(N_f)}{\tau_{axy}(N_f)}\right)^2 (1+\nu), \qquad (4.94)$$

where $\sigma_{axx}(N_f)$ and $\tau_{axy}(N_f)$ are fatigue strength under simple loading states (tension-compression (bending) and shearing (torsion), respectively) versus a number of cycles. These two fatigue characteristics are often parallel. Then, it is suitable to substitute the fatigue limit, and finally β from (4.94) is obtained

$$\beta = \left(\frac{\sigma_{\rm af}}{\tau_{\rm af}}\right)^2 (1+\nu). \tag{4.95}$$

Stage 5

At this stage, extrema of the energy parameter history are determined from (4.91) and (4.92), like in the case of stress histories.

Stage 6

As it was said, the rain flow method (the envelope method) allows to determine both cycles and half-cycles. They are determined by suitable envelopes (see Fig. 4.2). This method is programmed and cycles are counted by a computer. Each time, the amplitude and the mean value of a cycle or a half-cycle are determined. Numerical procedure of cycle counting has been shown in Fig. 4.3. The same procedure is proposed for the strain energy density parameter.

Stage 7

At this stage, cycle amplitudes of the energy parameter are transformed in relation to the occurring mean values of this parameter. Such model was presented in [77, 78, 79]; however, it is not applied and thus not discussed in is paper.

Stage 8

There are many hypotheses concerning accumulation of amplitudes of stress cycles and half-cycles. As in the case of (3.21), it is possible to propose fatigue damage accumulation for cycle amplitudes of the strain energy density parameter [55, 58, 59, 64, 65, 66, 102, 103, 105, 106, 116, 122, 136, 154, 179, 180] according to a general formula

$$S(T_{o}) = \begin{cases} \sum_{i=1}^{J} \frac{n_{i}}{b * N * (W_{af} / W_{ai})^{m_{w'}}} & \text{for } W_{ai} \ge aW_{af} \\ h \sum_{i=j+1}^{k} \frac{n_{i}}{N_{o} (W_{af} / W_{ai})^{m'_{iw}}} & \text{for } W_{ai} < aW_{af} \end{cases}$$
(4.96)

where:

a – coefficient allowing to include amplitudes below W_{af} in damage accumulation,

 $m^\prime_{\rm w}$ – coefficient of the S–N curve slope for the strain energy density parameter,

Waf - fatigue limit according to the strain energy density parameter,

 n_i - number of cycles with amplitude W_{ai} (two identical half-cycles form one cycle),

and the remaining notations - as in the stress model (3.20).

Stage 9

After determination of the damage degree at observation time T_o according to (4.96), fatigue life is calculated

$$T_{cal} = \frac{T_o}{S(T_o)}$$
(4.97)

or

$$N_{cal} = \frac{N_{block}}{S(N_{block})}$$
(4.98)

as for the uniaxial loading state.

5 An Example of Fatigue Life Evaluation Under Simple Loading

5.1 Fatigue Tests

Static properties of the considered materials are shown in Table 5.1. Their tests have been discussed in [224] and other papers. The tests were performed under uniaxial constant amplitudes, variable amplitudes of normal distribution (Gaussian spectrum) without and with overload under axial and bending loading with two stress ratios

$$R = \frac{\sigma_{\min}}{\sigma_{\max}},$$
(5.1)

under a symmetric cycle (R = -1) or pulsating loading (R = 0). Two kinds of welds were tested (Fig. 5.1). All the joints were made by one person. Sheets 1250 mm in length were joined with the GMAW method; 6–21 layers were put on, depending on the native material and its thickness. The specimens were cut with the plasma method, radii and transition angles in the notch root were determined for 12 specimens and 4 sections.

The theoretical notch coefficient for welded joints depends on the sheet thickness, the radius in the notch root and the angle of weld face inclination. Procedure of determination of theoretical notch coefficients is presented in many papers, for example [182, 183, 185].

Steel	E, GPa	R _m , MPa	R _{0.2} , MPa
S355N	206	560	378
S355M	206	524	422
S680Q	206	868	784
S960Q	206	1072	998

Table 5.1. Static properties of steels

The theoretical notch coefficient for butt welds (Fig. 5.2) can be determined from

$$K_{t} = \left[1 + b_{1}\left(\frac{t}{\rho}\right)^{b_{2}}\right]\left[1 + \left(a_{0} + a_{1}\sin\theta + a_{2}\sin^{2}\theta + a_{3}\sin^{3}\theta\right)\left(\frac{t}{\rho}\right)^{l_{1} + l_{2}\sin(\theta + l_{3})}\right],$$
(5.2)

The constants from (5.2) are presented in Table 5.2.

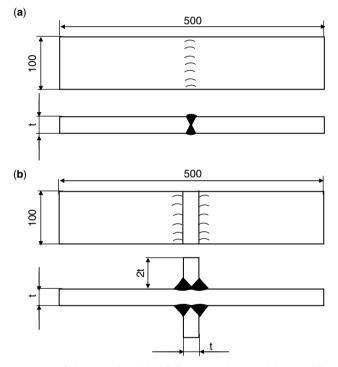


Fig. 5.1. Geometry of the tested welded joints: (a) butt welds, (b) fillet weld with the transverse stiffeners

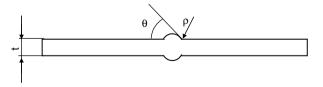


Fig. 5.2. Geometry of the butt welds joint for determination of the theoretical notch coefficient

Coefficient Loading	a _o	a ₁	a ₂	a ₃	b ₁	b ₂	l ₁	l ₂	l ₃
axial	0.169	1.503	-1.968	0.713	-0.138	0.213	0.249	0.356	6.194
bending	0.181	1.207	-1.737	0.689	-0.156	0.207	0.292	0.349	3.283

 Table 5.2. Coefficients in (5.2) depending on loading [8]

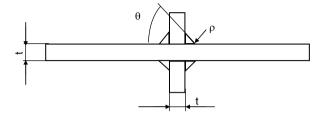


Fig. 5.3. Geometry of the fillet weld with the transverse stiffeners for determination of the theoretical notch coefficient

For the fillet joint with the transverse stiffeners (Fig. 5.3), the theoretical notch coefficient can be determined from the following equation

$$K_{t} = m_{o} + \left[1 + m_{2} \left(\frac{t}{\rho}\right)^{p_{3}} + m_{3} (\sin \theta)^{p_{4}}\right] (\sin \theta)^{p_{5}} \left(\frac{t}{\rho}\right)^{p_{6}}.$$
 (5.3)

The constants from (5.3) are given in Table 5.3.

Next, coefficients of stress concentration K_t were determined and after statistical processing they were presented in Table 5.4. The specimens of 30 mm thickness were tested under bending, and the specimens of 10 mm thickness were tested under axial loading (tension-compression).

Coefficient Loading	m _o	m ₂	m ₃	p ₃	p ₄	p ₅	p ₆
axial	1.538	1.455	-2.933	0.208	1.213	2.086	0.207
bending	1.256	12.153	-3.738	0.154	0.481	1.723	0.172

Table 5.3. Coefficients in (5.3) depending on loading

Steel	S355N	S355M	S690Q	S960Q
transverse stiffeners ($t = 10 \text{ mm}$)	4.78	3.67	3.79	4.27
butt welds $(t = 10 \text{ mm})$	2.06	2.47	2.69	2.22
transverse stiffeners ($t = 30 \text{ mm}$)	5.20	6.03	4.24	6.20
butt welds (t = 30 mm)	2.82	3.13	2.67	3.02

Table 5.4. Coefficients of stress concentration K_t

5.1.1 Tests Under Constant-amplitude Loading

After analysis of experimental data obtained under uniaxial cyclic loading, all the data obtained under axial loading and plane bending were collected in two separate groups. From the following equations

$$\sigma_{a} = K_{ta}\sigma_{an} \tag{5.4}$$

and

$$\sigma_{a} = K_{tb}\sigma_{an} \tag{5.5}$$

the amplitudes of pseudoelastic local stress for axial loading and plane bending are obtained, respectively. Transition from the nominal system to the local system using the theoretical coefficient of stress concentration has been shown in Fig. 2.6.

The ASTM standard [9] was applied for all the materials and both kinds of welded joints. As a consequence, the following regression equations S-N in the Basquin notation (3.2) for axial loading is obtained

$$\lg N = 11.390 - 2.280 \lg \sigma_a$$
 for $R = -1$ (5.6)

and

$$\lg N = 11.800 - 2.483 \lg \sigma_a$$
 for R=0. (5.7)

Table 5.5 presents test scatters for particular kinds of loading for the significance level $\alpha = 5\%$ and for two standard deviations. From analysis of the data in Table 5.5 it appears that the mean scatter band for cyclic tests has the coefficient close to 4. Interpretation of the data is shown in Figs. 5.4 and 5.5. From the figures it appears that all the materials both kinds of welded joints for particular stress ratios (R = -1) and (R = 0) can be approximated with use of one fatigue curve S–N.

No	Type of loading	R	sT _N	Τ _N (α=5%)	$T_{N}(2s)$
1	Axial loading	-1	1.919	3.878	3.838
2	Axial loading	0	2.133	4.266	4.266
3	Plane bending	-1	2.208	4.579	4.416
4	Plane bending	0	1.581	3.257	3.162
5	On the average		1.710	3.995	3.920

 Table 5.5. Scaters of fatigue tests in relation to the S–N curves for simple loading states

Here, sT_N is standard deviation of the scatters.

Similar analysis can be done for the plane bending

$$\lg N = 12.794 - 2.627 \lg \sigma_a$$
 for $R = -1$ (5.8)

and

$$\lg N = 13.304 - 2.835 \lg \sigma_a$$
 for R=0. (5.9)

Under axial loading from (5.6) and (5.7), the fatigue limit for pseudoelastic amplitudes of local stresses are $\sigma_{afa} = 172$ MPa and 176 MPa for R = -1 and R = 0 respectively for N = $2 \cdot 10^6$ cycles. Figures 5.6 and 5.7 show the

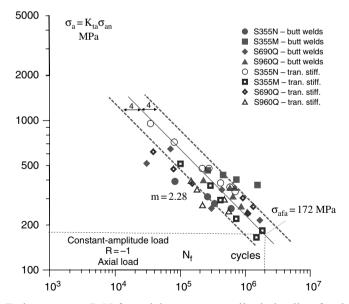


Fig. 5.4. Fatigue curves S–N for axial constant-amplitude loading for the considered materials and specimens under symmetric loading

test results for bending according to (5.8) (R = -1) and (5.9) (R = 0). Under plane bending from (5.7), The fatigue limit for pseudoelastic local stresses is σ_{afb} =296 MPa under N = 2 · 10⁶ cycles. Fatigue strength does not depend on the material, but on a loading type (axial or bending). In the local stress system, influence of geometry cannot be seen, either. From Fig. 5.4 – (5.6), (5.7), Fig. 5.6 – (5.8) and Fig. 5.7 – (5.9) it results that the mean stress does not influence the fatigue life, probably because of the existing high residual stress, found during other measurements shown in [224].

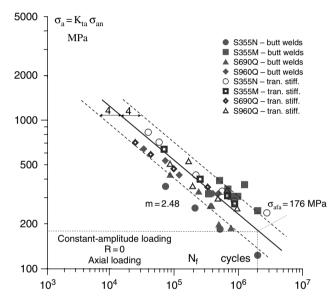


Fig. 5.5. Fatigue curves S–N for axial constant-amplitude loading for all the materials and specimens under pulsating loadings

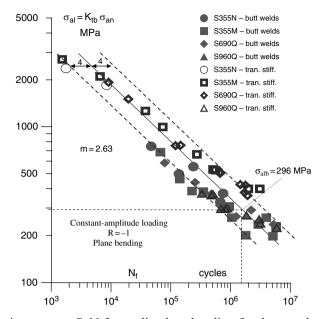


Fig. 5.6. Fatigue curves S–N for cyclic plane bending for the tested materials and specimens under symmetric loading

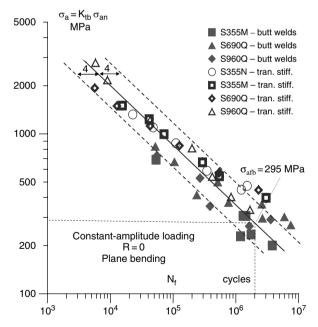


Fig. 5.7. Fatigue curves S–N for cyclic plane bending for the tested materials and specimens under pulsating loading

5.1.2 Tests Under Variable-amplitude Loading

In the case of Gaussian distribution of cyclic load amplitudes, the sequence length is $N_{block} = 5 \cdot 10^4$ cycles. In the case of the overload, overloads with a number of cycles $N_{OL} = 10^3$ are randomly distributed in the basic band, and the total spectrum length is $5 \cdot 10^4$ cycles, like for loading without overloads. As for the generated overloads, the ratio of their maximum values to the maximum value in the block of Gaussian loads is 1.4. Figure 5.8 shows a sequence of variable-amplitude loads with and without overloads versus the accumulated number of cycles n_{ic} . This sequence concerns normalized loads in Fig. 5.8a (symmetric loading) and Fig. 5.8b (pulsating loading).

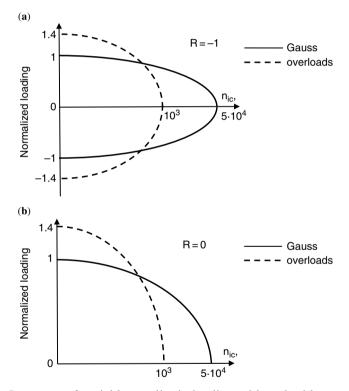


Fig. 5.8. Sequence of variable-amplitude loading with and without overloads, (a) symmetric loading, (b) pulsating loading versus the accumulated number of cycles n_{ic}

5.2 Verification of the Results Obtained Under Variable-amplitude Loading

Calculations were performed in order to compare calculation and experimental lives. The first such calculations were presented in [112, 228]. As it was said, large scatters of the fatigue test results were found. The scatters for cyclic tests were determined in the previous chapter. From the obtained data it appears that the scatter band of the cyclic test results has the coefficient 4. The calculated and experimental fatigue lives are presented in Figs. 5.9, 5.10 and 5.11. The calculation results are shown in Table 5.6. Many results are included into the scatter band with coefficient $T_N = 3$, but more of them are included into the scatter band with coefficient $T_N = 4$, but around the mean damage $\bar{T}_N = 1$. It means that the sum of most actual damages is included into the band

$$1/T_{\rm N} < \overline{\rm T}_{\rm N} \approx 1 < {\rm T}_{\rm N}. \tag{5.10}$$

It concerns more than 95% of the considered data.

			Сус	lic tests	Variable	-amplitude tests
No	Type of loading	R	\vec{T}_N	T _N (2s)	\bar{T}_N	T _N (2s)
1	Axial loading	-1	1	3.838	1.018	4.169
2	Plane bending	-1	1	4.416	1.143	4.385
3	Plane bending	0	1	3.162	1.449	3.750

Table 5.6. Scatters of the fatigue test results related to S–N curves for simple loading states

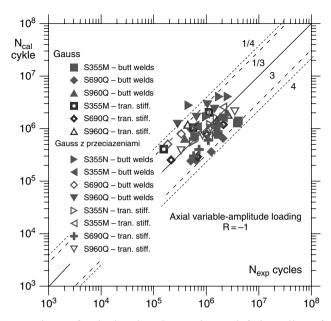


Fig. 5.9. Comparison of calculated and experimental fatigue lives for variableamplitude axial loading

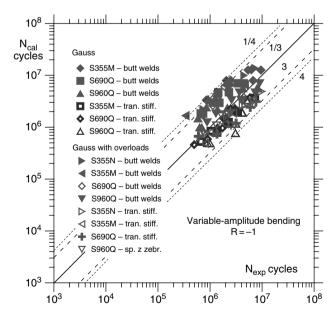


Fig. 5.10. Comparison of calculated and experimental fatigue lives for uniaxial variable-amplitude bending

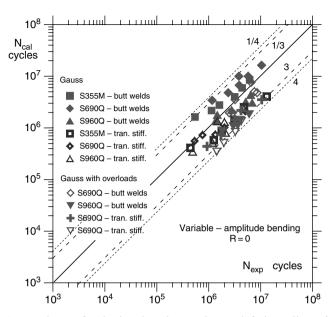


Fig. 5.11. Comparison of calculated and experimental fatigue lives for uniaxial variable-amplitude bending

6 An Example of Fatigue Life Evaluation Under Complex Loading States

6.1 Fatigue Tests

Experimental verification was based on fatigue tests performed by Sonsino [158, 165, 217, 220, 221, 227], Witt [240, 241, 242, 243] and Küppers [86, 87, 88, 113, 114, 226]. Table 6.1 presents mechanical properties of the considered materials (steel StE460 and aluminium alloy AlSi1MgMn T6). Figure 6.1 shows geometries of the tested specimens.

Welded joints were tested under pure bending, pure torsion and bending with torsion, in- and out-of-phase (90°). Under combined bending with torsion, a ratio of nominal shearing to bending stress was determined for tests performed by Sonsino and Küppers

$$\frac{\tau_{\rm an}}{\sigma_{\rm an}} = 0.58\tag{6.1}$$

and for tests by Witt

$$\frac{\tau_a}{\sigma_a} = \frac{K_{tt}\tau_{an}}{K_{tb}\sigma_{an}} = 0.6.$$
(6.2)

Material	Е	ν	R _{0.2}	R _m	A ₅
	GPa		MPa	MPa	%
StE460 (Sonsino)	206	0.30	520	670	25
StE460 (Witt)	192	0.30	466	624	28.7
AlSi1MgMn T6 (6082 T6) (Küppers)	71.3	0.32	315	332	13

Type of welded joint	ρ_{real}, mm	K _{tb}	K _{tt}
Flange-tube a (Sonsino)	0.45	3.93	1.85
Tube-tube (Sonsino)	0.45	2.42	1.77
Flange- tube (Witt)	no data	2.20	1.32
Flange-tube (Küppers)	17	1.62	1.14
Tube-tube (Küppers)	1.7	1.68	1.21

Table 6.2. The calculated radii and stress concentration factors for notches

where ρ_{real} is an actual mean radius in the notch root.

Analysis of tests performed by Sonsino was done for rough specimens, and in the case of Witt's tests machined specimens were considered. Thus, the notch coefficients for Witt's tests are lower than those for Sonsino's tests (see Table 6.2). Table 6.2 shows comparison of all the notch coefficients for fictitious radii and stress concentration factors for actual radii. All the coefficients were defined by calculations with the finite element method for particular radii and constant angle of the weld face [217].

The stress concentration factors included in Table 6.2 confirm the known relationship [219, 225, 249]

$$\mathbf{K}_{\mathrm{ta}} > \mathbf{K}_{\mathrm{tb}} > \mathbf{K}_{\mathrm{tt}}.\tag{6.3}$$

In the case of rough welded joints, the stress concentration factor is a function of the radius in the notch root. From calculations, the relationships for tube-flange joints for bending were obtained

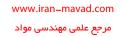
$$\log K'_{tb} = 0.505 - 0.267 \log \rho$$
(6.4)

and torsion

$$\log K'_{tt} = 0.215 - 0.151 \log \rho. \tag{6.5}$$

Similar relationships were obtained for tube-tube joints for bending and torsion, respectively

$$\log K'_{tb} = 0.299 - 0.235 \log \rho \tag{6.6}$$



Type of joint	ρ*, mm	ρ_{fb} , mm	K′ _{fb}	$\rho_{\rm ft}$, mm	K′ _{ft}
Flange-tube	4.0	1.16	3.11	0.40	1.88
	3.5	1.02	3.21	0.35	1.92
Tube-tube	4.0	1.16	1.92	0.40	1.79
	3.5	1.02	1.98	0.35	1.83

Table 6.3. The calculated fictitious radii and stress concentration factors for notches

and

$$\log K'_{tt} = 0.181 - 0.181 \log \rho. \tag{6.7}$$

Then, stress concentration factors were determined for bending and torsion, the substitute value of microstructure $\rho^* = 0.4$ mm was assumed. Such assumption is often made for constructional steels. Additional calculations were performed for $\rho^* = 0.35$ mm, according to Fig. 2.9. Next, according to (2.12) and the Huber-Mises-Hencky hypothesis, fictitious radii ρ_f were determined (see Table 6.3). From the data presented in Table 6.3 it appears that influence of $\rho^* = 0.35$ mm and 0.4 mm on determination of stress concentration factors is relatively small. Thus, the universal value $\rho^* = 0.4$ mm is recommended. From the same table it also results that different fictitious notch radii are obtained for bending and torsion.

After the recalculation of the values from the nominal system to the local system with the use of theoretical notch coefficients for bending and torsion a new S–N characteristics was obtained, i.e.

$$\log N_{f} = A - m \cdot \log \sigma_{a} \tag{6.8}$$

and

$$\log N_{\rm f} = A_{\tau} - m_{\tau} \cdot \log \tau_{\rm a}, \tag{6.9}$$

determined on the basis of the test results through regression analysis according to the ASTM standard [9]. The determined parameters of the fatigue curves S–N are given in Table 6.4, where m is inclination, and k is determined as

$$k(N_f) = \frac{\sigma_a(N_f)}{\tau_a(N_f)}.$$
(6.10)



Table 6.4. Parameters	of the	S-N	curves	in	the	local	system	for	tested .	joints
for $R = -1$										

Welded joint	А	m	r	A_{τ}	m _τ	r_{τ}	k
Flange-tube (Sonsino)	17.034	4.306	0.965	25.782	8.233	0.974	1.65*
Tube-tube (Sonsino)	16.342	4.207	0.968	_	_	_	1.65*
Sonsino - total	15.015	3.658	0.940	25.782	8.233	0.974	1.36*
Flange-tube (Witt)	16.199	4.227	0.982	24.033	7.435	0.958	1.74**
Flange-tube (Küppers)	15.615	5.124	0.918	14.598	5.159	0.897	1.62**
Tube-tube (Küppers)	17.324	5.541	0.847	19.895	8.107	0.390	2.23**

 $*N_f = 3 \cdot 10^5$ cycles $**N_f = 5 \cdot 10^5$ cycles

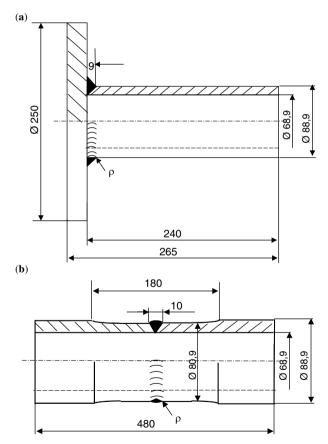


Fig. 6.1. Geometries of welded joints (a) flange-tube (FT), (b) tube-tube (TT)

All the experimental data for flange-tube and tube-tube specimens are shown in Figs. 6.2, 6.3, 6.4, 6.5, 6.6. According to the standards [38], a slope of the S–N curves m = 5, is typical for welded joints (see Sect. 1.1). As it can be seen in Table 6.4, under bending inclinations (m = 3.658-5.541) were obtained, and they were equal to the recommended inclination (m = 5). It was shown in the next figures as the reference scatter band for fatigue test results obtained under more complex loading. The obtained conformity shows whether the method of evaluation of the results obtained under complex loading is suitable for formulation the equivalent parameter under the fatigue damage.

Table 6.5 presents scatters of the fatigue test results related to the determined S-N curves for pure bending. In lines 1-5 there are scatters for particular fatigue tests. Line 6 presents the mean values calculated on the basis of scatters for all the tests. As for the Sonsino's tests, the total characteristic has been determined and it is shown in line 7. It can be seen that the mean scatter equal to 1 was obtained in all the cases. If theoretical notch coefficients based on the fictitious notch radius in the welded joint coincide with the fatigue one (line 8), then the mean scatters for flange-tube and tube-tube joints (line 9) are equal to 1 in relation to characteristics determined from the total characteristic of the considered welded joints. The mean standard deviation sT_N is 1.65, two standard deviations are $2sT_N = 3.331$. Let us notice a similarity with the often assumed scatter band band with the coefficient 3. For the significance level $\alpha = 5\%$, the mean scatter is $T_N = tsT_{N5\%/2} = 3.748$. Lower scatters can be observed for steel joints, and higher scatters are for aluminium joints. The scatters at the level 2sT_N are shown in Figs. 6.2, 6.3, 6.4, 6.5, 6.6, 6.7. All the calculation results are compared with those scatters.

From the calculation results presented in Table 6.5 it appears that in a complex case or under variable-amplitude loading the considered algorithm can be assumed as correct, if the calculation results are included into these scatter bands.

No	Welded joint	\overline{T}_{N}	sT_N	T _N (α=5%)	$T_N (2sT_N)$
1	Flange-tube (Sonsino)	1	1.641	3.878	3.282
2	Tube-tube (Sonsino)	1	1.455	3.101	2.910
3	Flange-tube (Witt)	1	1.492	3.172	2.992
4	Flange-tube (Küppers)	1	1.675	4.099	3.350
5	Tube-tube (Küppers)	1	2.061	4.491	4.121
6	All the joints together	1	1.665	3.748	3.331
7	Sonsino – total	1	1.758	3.687	3.565
8	Flange-tube (Sonsino) related to total	1.738	1.652	4.107	3.304
9	Tube-tube (Sonsino) related to total	1.282	1.521	2.732	3.042

Table 6.5. Scatters of the fatigue test results related to S-N curves for bending

Figures 6.7 and 6.8 show the weighed amplitudes of m degree for random tests of steel and aluminium welded joints [107, 157]. The amplitudes expressed by the following equation are most often applied by researchers

$$\sigma_{aw} = \left[\frac{\sum_{i=1}^{n} \sigma_{ai}^{m} n_{i}}{\sum_{i=1}^{n} n_{i}}\right]^{1/m}.$$
(6.11)

From Fig. 6.7 it appears that the weighed amplitudes for steel welded joints under bending are included into the scatter band for cyclic bending. From Fig. 6.8 it results that the weighed amplitudes for welded aluminium joints under bending are not included into the scatter bend for cyclic bending. Thus, the simple Palmgren-Miner rule is probably not valid in such a case.

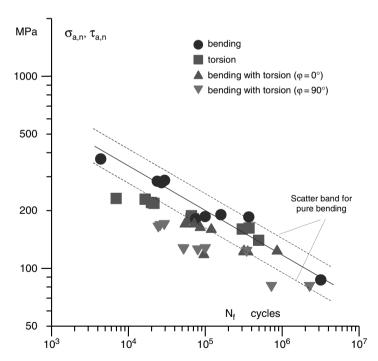


Fig. 6.2. Experimental data for flange-tube welded joints FT according to tests by Sonsino [217]

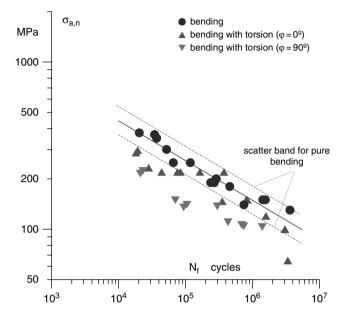


Fig. 6.3. Experimental data for tube-tube welded joints TT according to tests by Sonsino [217]

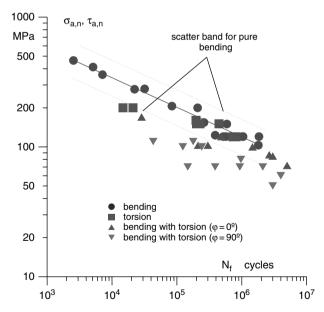


Fig. 6.4. Experimental data for flange-tube welded joints FT according to tests by Witt [240]

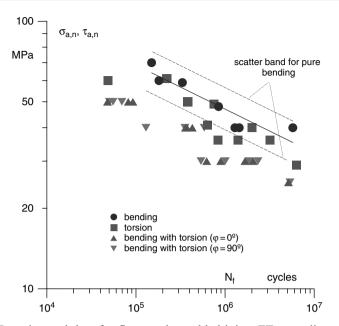


Fig. 6.5. Experimental data for flange-tube welded joints FT according to tests by Küppers [86, 88, 226]

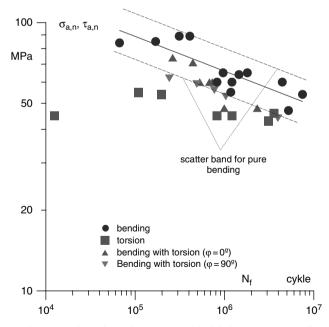


Fig. 6.6. Experimental data for tube-tube welded joints TT according to tests by Küppers [86, 88, 226]

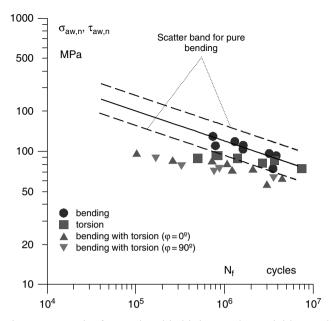


Fig. 6.7. Fatigue test results for steel welded joints under variable-amplitude loading according to Witt FT [104]

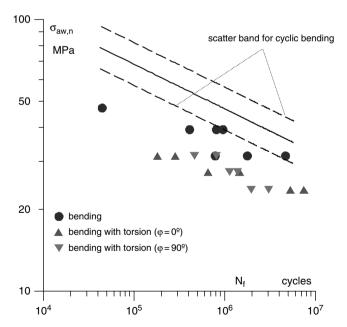


Fig. 6.8. Fatigue test results for aluminium welded joints under variable-amplitude loading according to Küppers FT [87]

Lp.	Type of loading	Calculations related to the S-N curve	Notation	Tests
1	Flange-tube	Flange-tube	FT	Sonsino Witt Küppers
2	Tube-tube	Tube-tube	TT	Sonsino Küppers
3	Flange-tube	Total characteristics	FTc	Sonsino
4	Tube-tube	Total characteristics	TTc	Sonsino

Table 6.6. The applied abbreviations

6.2 Verification of the Criteria Under Constant-amplitude Loading

The proposed criteria have been verified and the results of verification have been presented here. Some abbreviations have been introduced (Table 6.6).

6.2.1 The Parameter of Shear and Normal Strain Energy Density on the Critical Plane Determined by the Parameter of Normal Strain Energy Density

In the following subchapter the verification of the criterion assuming that the plane determined by maximum parameter of normal strain energy density in the critical plane is presented. Then, in this plane a sum of the normal strain energy density parameter (with the weight coefficient 1) and the shear strain energy density parameter in this plane is determined. As it was said, the weight (coefficient β) that should be assumed for the shear strain energy density parameter has not been determinated. Thus, analysis of the scatter \overline{E} was performed for steel welded joints, determined according to (3.33) for all non-proportional tests and different values of the coefficient β . Values of scatters \overline{E} versus coefficient β including the shear strain energy density parameter for steel welded joints are shown in Fig. 6.9. From this figure it appears that a value of the coefficient β varies within 7–14, depending on the tests. Its average value of 10 can be assumed.

Figures 6.11–6.17 show the comparison of calculated and experimental lives for particular tests and for coefficients β determined from Fig. 6.9 including the shear strain energy density. It is important to note that for the tests of tube-flange joints performed by Sonsino (Fig. 6.10), the scatter is included into the band for pure bending except for torsion. When calculations are related to the total characteristics (Fig. 6.11), the calculated lives decrease and exceed the scatter band for proportional and one point for non-proportional loading are outside the scatter band for pure bending. It is important to note that for tube-tube joints there are no tests under pure torsion because the cracks occur outside the joint in the native material. When calculations are related to the total fatigue characteristics (Fig. 6.13) similar results have been obtained. As for tests by Witt (Fig. 6.14), the tests results are similar as those obtained by Sonsino (see Fig. 6.10).

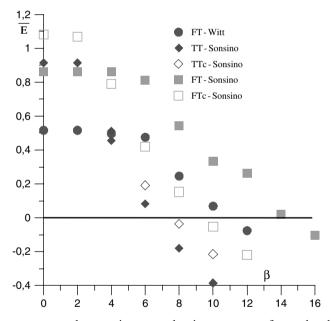


Fig. 6.9. Scatters versus shear strain energy density parameters for steel welded joints

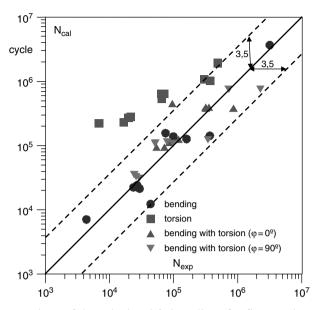


Fig. 6.10. Comparison of the calculated fatigue lives for flange-tube welded joints (FT) according to the criterion in the plane determined by the normal strain energy density parameter with the lives obtained by Sonsino

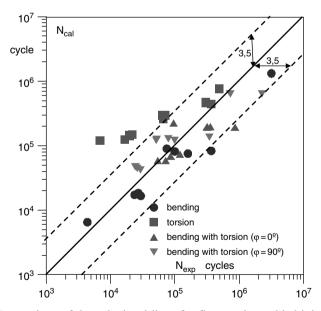


Fig. 6.11. Comparison of the calculated lives for flange-tube welded joints (FTc) according to the criterion in the plane determined by the normal strain energy density parameter with the lives obtained by Sonsino related to the total fatigue characteristics

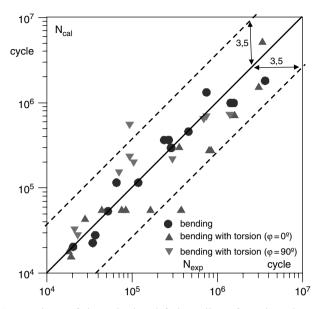


Fig. 6.12. Comparison of the calculated fatigue lives for tube-tube welded joints (TT) according to the criterion in the plane determined by the normal strain energy density parameter with the lives obtained by Sonsino

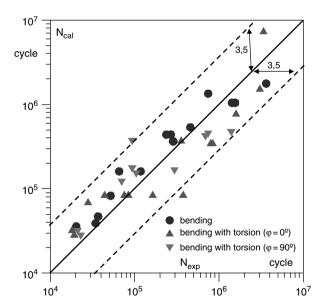


Fig. 6.13. Comparison of the calculated fatigue lives for tube-tube welded joints (TTc) according to the criterion in the plane determined by the normal strain energy density parameter with the lives obtained by Sonsino related to the total fatigue characteristics

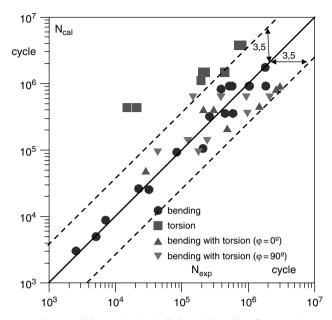


Fig. 6.14. Comparison of the calculated fatigue lives for flange-tube welded joints (FT) according to the criterion in the plane determined by the normal strain energy density parameter with the lives obtained by Witt for equal frequencies of bending with torsion

In the case of the tests of aluminium joints performed by Küppers, overestimated calculated fatigue lives were obtained for both tube-flange and tube-tube joints (Figs. 6.15 and 6.16). That overestimation can be even 100 times higher than the experimental value. During these tests, the values of the coefficient β , including participation of the shear strain energy density parameter in the critical plane, were not searched because favourable results had not been obtained for proportional loading.

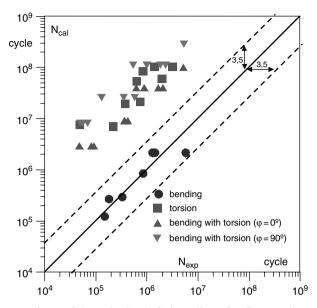


Fig. 6.15. Comparison of the calculated fatigue lives for flange-tube welded joints (FT) according to the criterion in the plane determined by the normal strain energy density parameter with the lives obtained by Küppers

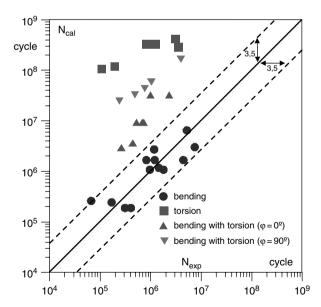


Fig. 6.16. Comparison of the calculated fatigue lives for tube-tube welded joints (TT) according to the criterion in the plane determined by the normal strain energy density parameter with the lives obtained by Küppers

6.2.2 The Parameter of Shear and Normal Strain Energy Density in the Critical Plane Determined by the Shear Strain Energy Density Parameter

In this part the criterion assuming the plane determined by the maximum value of shear strain energy density parameter as the critical plane was verified. In this plane, a sum of the normal strain energy density parameter and the shear strain energy density parameter is determined.

Figures 6.17-6.23 present comparisons of the calculated and experimental fatigue lives for particular tests. After the analysis of the experiment and the calculations it can be stated that in the case of the tests performed by Sonsino for tube-flange joints (Fig. 6.17) the results are included into the scatter band for pure bending, except for one point for proportional loading. If the calculation results are related to the total characteristics (Fig. 6.18), the calculated lives decrease like in the case when the plane of the normal strain energy density parameter was assumed as the critical plane (see Figs. 6.10 and 6.11). As for tube-tube welded joints (Fig. 6.19) it is possible to find that only one point for proportional loadings and one point for non-proportional loadings are located outside the scatter band for pure bending, like in the case when the plane determined by the normal strain energy density parameter is assumed as the critical plane (Fig. 6.12). Similar results were obtained for the tests related to the total fatigue characteristics (Fig. 6.20) and in Fig. 6.13. During the tests performed by Witt, when the plane of the shear strain energy density parameter was assumed as the critical plane (Fig. 6.21), a little poorer results were obtained as compared with the case when the plane determined by the normal strain energy density parameter was assumed as the critical plane (Fig. 6.14). It concerns, however, only the stress levels close the fatigue limit, where scatters of experimental results are greater. In the case of torsion, it can be stated that the calculated results from Fig. 6.21 are better than those in Fig. 6.14.

As for the tests by Küppers for aluminium welded joints, much better results were obtained than in the case when the plane of the normal strain energy density parameter was assumed as the critical plane for tube-flange joints (Fig. 6.22) and tube-tube joints (Fig. 6.23). Only some points are located outside the scatter band for pure bending.

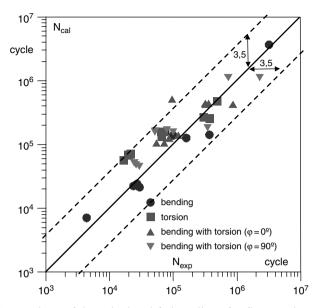


Fig. 6.17. Comparison of the calculated fatigue lives for flange-tube welded joints (FT) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Sonsino

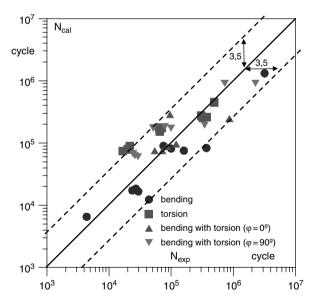


Fig. 6.18. Comparison of the calculated fatigue lives for flange-tube welded joints (FTc) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Sonsino in relation to the total fatigue characteristics

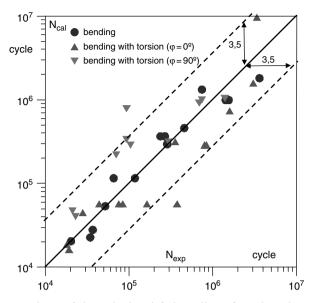


Fig. 6.19. Comparison of the calculated fatigue lives for tube-tube welded joints (TT) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Sonsino

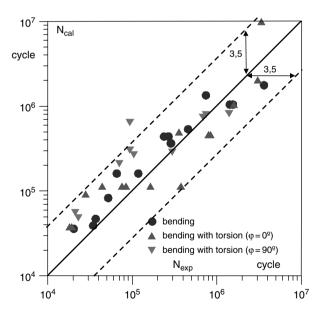


Fig. 6.20. Comparison of the calculated fatigue lives for tube-tube welded joints (TTc) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Sonsino related to the total fatigue characteristics

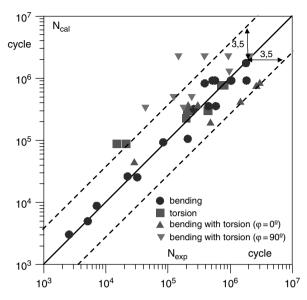


Fig. 6.21. Comparison of the calculated fatigue lives for flange-tube welded joints (FT) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Witt under equal frequencies of bending and torsion

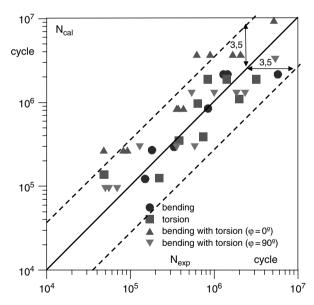


Fig. 6.22. Comparison of the calculated fatigue lives for flange-tube welded joints (FT) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Küppers

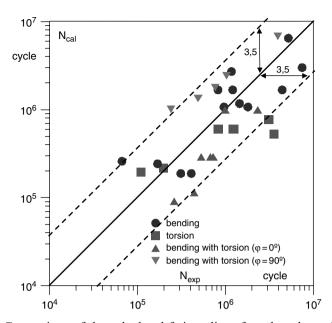


Fig. 6.23. Comparison of the calculated fatigue lives for tube-tube welded joints (TT) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Küppers

A qualitative (not quantitative) description was used for analysis of the results shown in Figs. 6.10-6.23. Thus, according to the suggestions included into Sect. 3.2, the mean scatters \overline{T}_N were determined for lives and the scatter bands T_N were determined under a double standard deviation. The calculation results are shown in Table 6.7. From the data presented in this table it appears that if the plane determined by the maximum shear strain energy density parameter is assumed as the critical plane, less mean scatters and scatter coefficients are obtained in comparison with the case when the plane determined by the maximum normal strain energy density parameter is understood as the critical plane. It is evident in the case of the tests by Küppers for welded aluminium joints. Only in one case (the tests by Sonsino for tube-tube welded joints) a contrary effect can be observed. However, the differences are small for steel welded joints for both considered critical planes. Thus, in this case one of two possible critical planes can be assumed: the plane determined by the normal strain energy density parameter (like for cast irons), or by the shear strain energy density parameter (like for steels). Here, it should be noted that the weld structure has a brittle material character, like for example, cast iron.

Critical			Sonsino			Witt	Küppers	
plane		FT	TT	FTc	TTc	FT	FT	TT
Wη	\overline{T}_N	2.046	1.208	1.236	1.106	1.683	40.365*	64.565*
·· η	T _N	3.936	3.936	5.047	3.900	7.835	3.656*	10.790*
$W_{\eta s}$	\overline{T}_{N}	1.538	1.005	1.439	1.340	1.549	1.479	1.330
···ηs	T _N	3,162	4,477	4,266	3,954	5,673	4,083	5,129

Table 6.7. Scatters of the cyclic test results according to the selected criteria

* only proportional loadings

If the plane determined by the maximum normal strain energy density parameter is assumed as the critical plane, after non-proportional tests, the value of the coefficient β including a part concerning the shear strain energy density parameter in the expression for the equivalent strain energy parameter should be determined. Thus, it is much better to apply the energy criterion based on the critical plane determined by the shear strain energy density parameter. This criterion was used for further calculations.

After the comparison of the scatter bands for uniaxial cyclic tests under pure bending (Table 6.5), when the mean scatter band varies about 3.5 and under combined bending with torsion with constant frequencies (Table 6.7) it can be seen that under complex loading the scatter band increase in relation to simple loadings, except for the tests by Sonsino for flange-tube (FT) welded joints.

6.2.3 The Influence of Different Frequencies of Bending and Torsion on Fatigue Life

Witt performed also tests under different frequencies of bending, f_{σ} and torsion, f_{τ} under the following combinations

 $f_{\sigma} = 5f_{\tau} \tag{6.12}$

and

$$\mathbf{f}_{\sigma} = 0.2\mathbf{f}_{\tau}.\tag{6.13}$$

The calculated and experimental results for all the cyclic tests are compared in Fig. 6.24, where the assumed number of cycles comes from bending. For those tests, the mean scatter $\overline{T}_N = 1.306$ and the scatter band with coefficient $T_N = 4.667$, were obtained. The obtained scatter band is less than that for tests for equal frequencies (Table 6.7), and greater than that

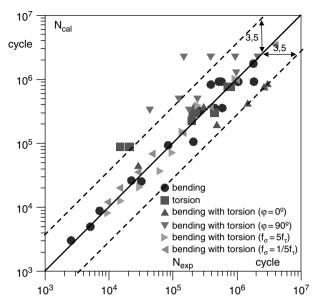


Fig. 6.24. Comparison of the calculated fatigue lives for flange-tube welded joints (FT) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Witt under different frequencies of bending and torsion

for pure bending (Table 6.5). From the analysis of results it appears that for loadings with different frequencies the obtained conformity of calculation and the experiment was very good.

6.3 Verification Under Variable-amplitude Loading

The reported variable-amplitude tests concerned flange-tube welded joints according to Witt, and aluminium joints according to Küppers.

The results for steel welded joints are shown in Fig. 6.25. The weighed amplitude σ_{aw} according to (6.11) is expressed by

$$\sigma_{\rm aw} = 0.370 \ \sigma_{\rm amax}. \tag{6.14}$$

The determined mean scatters are $\overline{T}_N = 1.268$, and the scatter band is $T_N = 3.846$. The scatters are greater than those for pure bending (Table 6.5), but lesser than those for combined bending with torsion (Table 6.7).

Good results of calculations were obtained as compared with the experimental data.

The results for welded aluminium joints are presented in Fig. 6.26. The weighed amplitude, σ_{aw} according to (6.11), is expressed as

$$\sigma_{\rm aw} = 0.393 \ \sigma_{\rm amax}. \tag{6.15}$$

The calculated results for pure bending are overestimated in relation to the experimental results. While searching a description of this phenomenon, the correction coefficient was applied, like in the Serensen-Kogayev hypothesis. The correction coefficient can be expressed as

$$b' = \frac{\sigma_{aw}}{\sigma_{a \max}}.$$
(6.16)

According to (6.15) and (6.16), b'= 0.393, which means that it is less than one and close to 1/3. Next calculations of life were performed according to

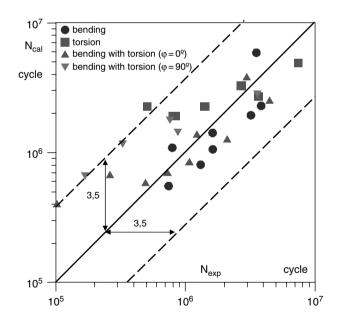


Fig. 6.25. Comparison of the calculated and experimental fatigue lives for flangetube welded joints (FT) under variable-amplitude loading according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Witt

that modification of the Palmgren-Miner hypothesis. The determined mean scatters were $\overline{T}_N = 1/0.940 = 1.063$, and the scatter band was $T_N = 4.519$, and in this case the scatters are less than those for tests under pure bending (Table 6.5) and under combined bending with torsion (Table 6.7). The differences are not very high, so it can be assumed that the obtained results are satisfactory.

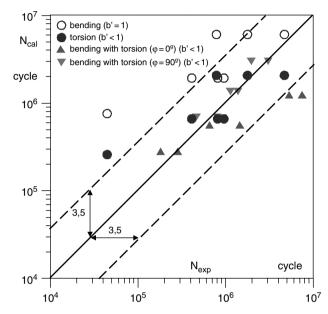


Fig. 6.26. Comparison of the calculated fatigue lives under variable-amplitude loadings for flange-tube welded joints (FT) according to the criterion in the plane determined by the shear strain energy density parameter with the lives obtained by Küppers

7 Conclusions

On the basis of the performed analyses and calculations the following conclusions can be drawn:

1. In the case of tests under uniaxial cyclic loading it can be stated that:

- 1.1 It is possible to determine one common fatigue characteristics for four considered materials.
- 1.2 Fatigue strength does not strongly depend on a type of the considered welded joint; much greater is the influence of loading (bending and axial loading)
- 1.3 The calculated sum of damages for normal distribution (Gaussian spectrum) and normal distribution with overloads are included into the scatter band with coefficient 3 according to the Palmgren-Miner hypothesis and with the significance level 5%.
- 1.4 In the considered joints, the mean stress values do not influence fatigue life; the obtained fatigue curves for symmetric and pulsating loadings were almost identical. It probably results from the existing high residual stresses, previously measured.
- 2. In the case of multiaxial loading it can be concluded that:
 - 2.1 Before estimation of multiaxial fatigue history in welded joints by local stresses and strains, the actual local radius at the weld edge should be determined. Owing to the fictitious local radius, what in the worst case for sharp notches $\rho=0$ means a crack, the notch coefficients for bending, K_{fb} and for torsion, K_{ft} can be calculated. Therefore, it is necessary to define the fictitious notch radii ρ_f for bending and torsion separately. In the case of welded steel joints, the radii are $\rho_{fb} = 1.16$ mm for bending and $\rho_{ft} = 0.4$ mm for torsion.
 - 2.2 The normal and shear strain energy density parameters in the critical plane determined by the parameter of shear and normal strain energy density for steel welded joints give comparable results. If the plane determined by normal strain energy density parameter is assumed as the critical plane, it is necessary to define the experimental weight function including the shear strain energy density parameter in this plane. Thus, the application of

the energy criterion defined in the plane determined by the shear strain energy density parameter is recommended.

- 2.3 In the case of welded aluminium joints, satisfactory results of fatigue life calculations were obtained for the criterion of energy parameter in the plane defined by the shear strain energy density parameter. When the energy criterion was applied in the plane defined by the normal strain energy density parameter, the calculated results were overestimated in comparison with the experimental ones.
- 2.4 Application of the maximum shear and normal strain energy density parameter in the critical plane for aluminium welded joints subjected to variable-amplitude bending with torsion is right if the Palmgren-Miner hypothesis is taken into account, and the correction coefficient is used, like in case of the Serensen-Kogayev hypothesis. The correction coefficient is the quotient of the weighed amplitude of the slope degree of the fatigue curve and the maximum amplitude.

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Summary

The paper presents fatigue life calculations for some chosen welded joints. The results were verified on the basis of fatigue tests of steel and aluminium welded joints under uniaxial and multiaxial loading states. Uniaxial loading concern pure tension-compression and alternating bending under cyclic and random tests of specimens made of steel. One fatigue characteristic can be determined for four considered materials. From the calculations it appears that fatigue strength does not strongly depend on a type of the considered welded joint – it is more dependent on loading type (bending and axial loading). The calculated sum of damages for normal distribution (the Gaussian spectrum) and normal distribution with overloads is included into the scatter band with coefficient 3 according to the Palmgren-Miner hypothesis at the significance level 5%. In the case of the considered welded joints, the mean stress value does not influence the fatigue life. The same fatigue curves have been obtained for symmetric and pulsating loading.

Complex stress states concern loading under combined proportional and non-proportional cyclic bending with torsion. Moreover, for aluminium joints verification was also done under random loading. Evaluating the multiaxial fatigue histories in welded joints by local stresses and strains, we must know the actual local radius at the weld edge. Owing to the fictitious local radius, when - in the worst case - for sharp notches $\rho = 0$, it is possible to calculate coefficients of the notch action for bending, K_{th} and for torsion, K_{ft} . In this order we must determine fictitious radii of the notch ρ_f for bending and for torsion. In the case of steel welded joints, these radii are $\rho_{\rm fb} = 1.16$ mm for bending and $\rho_{\rm ft} = 0.4$ mm for torsion. The normal and shear strain energy density parameter in the critical plane determined by the energy density parameter of normal and shear strain for steel welded joints gives comparable results. However, if the normal strain energy density parameter is assumed as the critical plane, it is necessary to determine, in an experimental way, the weight function including the shear strain energy density parameter in this plane. Thus, application of the energy criterion defined in the plane determined by the shear strain energy density parameter, is recommended. In the case of aluminium welded joints, satisfactory results of fatigue life calculations were obtained for the criterion of energy parameter

in the plane defined by the shear strain energy density parameter. In the case of application of the energy criterion in the plane defined by the normal strain energy density parameter, the obtained calculated fatigue lives were strongly overestimated in comparison of the experimental results. Application of the maximum shear and normal strain energy density parameter in the critical plane for aluminium welded joints subjected to variableamplitude bending with torsion seems to be right under the Palmgren-Miner hypothesis and application of the correction coefficient, like in the case of the Serensen-Kogayev hypothesis, which is the quotient of the weighed amplitude of the fatigue curve inclination in energy approach and the maximum amplitude in the history.